







GEOTECHNICAL AND PEAT STABILITY ASSESSMENT REPORT

DERRINLOUGH WIND FARM, CO. OFFALY

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- Keywords: Geotechnical, Peat Stability, Peat Failure, Ground Investigation, Risk Assessment
- **Abstract:** Fehily Timoney and Company (FT) were engaged by McCarthy Keville O'Sullivan to undertake a geotechnical assessment of the proposed Derrinlough wind farm site with respect to peat stability. As part of the geotechnical assessment of the proposed development, FT completed walkover surveys at the site and a ground investigation comprising 68 no. trial pits with associated laboratory testing. The findings of the geotechnical and peat stability assessment showed that the site has an acceptable margin of safety and is suitable for the proposed wind farm development.

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1 NON-TECHNICAL SUMMARY

Fehily Timoney and Company (FT) formerly Applied Ground Engineering Consultants Ltd (AGEC) was engaged by McCarthy Keville O'Sullivan on behalf of Bord na Móna Powergen Ltd to undertake a geotechnical and peat stability assessment of the proposed Derrinlough wind farm site. In accordance with planning guidelines compiled by the Department of the Environment, Heritage and Local Government (DoEHLG), where peat is present on a proposed wind farm development, a peat stability assessment is required.

The findings of the peat assessment, which involved analysis of 579 no. locations, showed that the site has an acceptable margin of safety and is suitable for the proposed wind farm development. The findings include recommendations and control measures for construction work in peat lands to ensure that all works adhere to an acceptable standard of safety.

The proposed wind farm comprises 21 no. wind turbines and associated infrastructure.

The site is relatively flat-lying with drainage channels running typically southeast to northwest. The site is split into two areas, Drinagh to the east of the N62 national road and Clongawny to the west. The land uses and types within the proposed development site are a mixture of bare cutover and cutaway peat, revegetation of bare peat, commercial forestry, telecommunications (a 30m Mast) and wind measurement (a single 100m anemometry mast on Clongawny Bog). There are also a number of Bord na Móna rail lines that pass through the bogs facilitating the transportation of milled peat to Derrinlough Briquette Factory which is located in the most western part of Drinagh bog.

Bord na Móna has considerable experience in the handling of peat in these circumstances, both during peat production operations and during wind farm construction projects. This experience has shown that the most environmentally sensitive and stable way of handling and moving of peat is its placement across the site and at locations as close as possible to the excavation areas.

Peat thicknesses recorded during the FT walkover, ground investigation and from the ground penetrating radar surveys carried out by Bord na Móna within the proposed infrastructure footprint on site ranged from 0 to 4.7m with an average of 1.1m. It should be noted that the peat thickness within the proposed infrastructure footprint is generally less than 2m with localised deeper deposits of up to 4.7m. The deeper peat areas were generally avoided when optimising the wind farm layout for site.

Slope inclinations at the main infrastructure locations range from 0 to 4 degrees. The flat topography/nature of the terrain on site highlights the low risk of peat failure.

Ground conditions comprised mainly of peat overlying typically lacustrine soil or till.

A walkover including intrusive peat depth probing, a ground investigation including trial pits, desk study, stability analysis and risk assessment was carried out to assess the susceptibility of the site to peat failure following the principles in Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments (PLHRAG, 2017).

The purpose of the stability analysis is to determine the stability i.e. Factor of Safety (FoS), of the peat slopes. The FoS provides a direct measure of the degree of stability of a peat slope. A FoS of less than 1.0 indicates that a slope is unstable; a FoS of greater than 1.0 indicates a stable slope. An acceptable FoS for slopes is generally taken as a minimum of 1.3.

From the stability analysis for both the undrained and drained conditions, which analysed the turbine locations and other proposed infrastructure locations, the calculated values were above the minimum acceptable FoS of 1.3 at 576 no. of the 579 no. locations. 3 no. locations were calculated with FoS's of between 1.08 and 1.29. The 3 no. marginally low FoS's are located alongside proposed access roads between turbines T12 and T13 and one location along the proposed access road close to turbine T4. The proposed works at these locations entails the construction of a floated section of access road i.e. no excavation works are proposed at these locations. Peat instability at these locations is not envisaged to be an issue.

As presented above for both the undrained and drained conditions, 100% of calculated FoS's are above 1.0 and 99.5% of calculated FoS's are above 1.3 which highlights the low risk of peat failure on site.

The risk assessment uses the results of the stability analysis in combination with qualitative factors, which cannot be reasonably included in a stability calculation but nevertheless may affect the occurrence of peat instability to assess the risk of peat failure at the site. The results of the risk assessment are given in Appendix B. A construction buffer zone plan based on qualitative factors identified during the site walkover is included as Figure 4-2.

The presence, or otherwise, of relict peat failures or clustering of relict failures within an area is an indicator that particular site conditions exist that pre-dispose a site to failure or not as the case may be. Based on the historical data reviewed and the terrain and ground conditions present on site it can be concluded that site conditions in the area of the Derrinlough site have low potential of peat failure.

In summary, the Derrinlough wind farm site has an acceptable margin of safety and is considered to be at **low** risk of peat failure.

2 INTRODUCTION

2.1 Background and Experience

Fehily Timoney and Company (FT) formerly Applied Ground Engineering Consultants Ltd (AGEC) was engaged in March 2018 by McCarthy Keville O'Sullivan on behalf of Bord na Móna to undertake a geotechnical and peat stability assessment of the proposed Derrinlough wind farm site.

FT have been involved in over 100 wind farm developments in both Ireland and the UK at various stages of development i.e. preliminary feasibility, planning, design, construction and operational stage and have established themselves as one of the leading engineering consultancies in peat stability assessment, geohazard mapping in peat land areas, investigation of peat failures and site assessment of peat.

The relevant geotechnical project team along with their relevant experience are outlined below.

- Gerry Kane (BEng, PGradDip, CEng, MIEI) is a Chartered Civil/Geotechnical Engineer with over 10 years geotechnical consultancy experience in Ireland and the UK. Gerry has completed numerous geotechnical & geological impact assessments for wind farm developments in Ireland. In addition, he has significant experience in the geotechnical/civil design of wind energy projects at construction stage.
- Ian Higgins (BSc, MSc, FGS, MIEI) is a Geotechnical Engineer with over 20 years geotechnical consultancy experience in Ireland and the UK. Ian has completed numerous geotechnical & geological impact assessments for wind farm developments in Ireland. In addition, he has significant experience in the geotechnical/civil design of wind energy projects at construction stage.
- Paul Jennings (PhD, BEng, DipArb, CEng, MIEI) is a Chartered Geotechnical Engineer with over 30 years geotechnical consultancy experience in Ireland, the UK and Hong Kong. Paul has completed numerous geotechnical & geological impact assessments for wind farm developments in Ireland. In addition, he has attended and represented developers and consultants at numerous oral hearings for wind energy developments.

2.2 Description of Works

The Derrinlough wind farm site comprises cut-away blanket peat area of approximately 24km². The site is located in the west of Co. Offaly. The nearest village to the site is Cloghan which is at about a distance of 2.5km. The surrounding landscape is predominately flat with land-use comprising forestry, agricultural land and cutaway peatland.

The proposed development comprises the following:

- (1) 21 No. wind turbines with an overall blade tip height of up to 185 metres and all associated hardstanding areas.
- (2) 2 No. permanent Anemometry Masts up to a height of 120 metres.
- (3) Provision of new and upgraded internal site access roads, passing bays, amenity pathways, amenity carpark and associated drainage.
- (4) 2 No. permanent underpasses in the townland of Derrinlough. One underpass will be located beneath the N62 and one will be located beneath an existing Bord na Móna rail line.
- (5) 1 No. 110 kV electrical substation, which will be constructed in the townland of Cortullagh or Grove. The electrical substation will have 2 No. control buildings, associated electrical plant and equipment and a wastewater holding tank.
- (6) 5 No. temporary construction compounds, in the townlands of Clongawny More, Derrinlough, Derrinlough/Crancreagh, Drinagh and Cortullagh or Grove.
- (7) All associated underground electrical and communications cabling connecting the turbines to the proposed electrical substation.
- (8) 2 No. temporary security cabins at the main construction site entrances in the townland of Derrinlough.

- (9) All works associated with the connection of the proposed wind farm to the national electricity grid, which will be to the existing Dallow/Portlaoise/Shannonbridge 110 kV line.
- (10) Removal of existing meteorological mast.
- (11) Upgrade of existing access and temporary improvements and modifications to existing public road infrastructure to facilitate delivery of abnormal loads including locations on the N52 and N62; construction access for delivery of construction materials at locations on the N62 and R357; operational access onto L7009 in the townland of Cortullagh or Grove and amenity access off R357 and L7005.
- (12) All associated site works and ancillary development including signage.
- (13) A 10-year planning permission and 30-year operational life from the date of commissioning of the entire wind farm.

The peat depth data recorded by FT during the site walkovers from the 22nd to the 23rd August 2019 and the 10th to the 11th September, by Hydro Environmental Services (HES) in 2019 has been used in the assessment of peat stability for the proposed wind farm site.

Ground investigation in the form of trial pits were carried out by FT/AGEC and Bord na Móna during the following dates:

- 29th and 30th January 2019
- 12th to 16th August 2019
- 7th and 8th October 2019
- 6th November 2019

In addition, a ground penetrating radar (GPR) survey of the site was carried out by Bord na Móna in 2008 and 2015. This peat depth data has also been used in the peat stability assessment.

2.3 Peat Stability Assessment Methodology

FT undertook the assessment following the principles in Peat Landslide Hazard and Risk Assessments: Best Practice Guide for Proposed Electricity Generation Developments (PLHRAG, 2017). The Peat Landslide Hazard and Risk Assessment Guide (PLHRAG) is used in this report as it provides best practice methods to identify, mitigate and manage peat slide hazards and associated risks in respect of consent applications for electricity generation projects.

The best practice guide was produced following peat failures in the Shetland Islands, Scotland in September 2003 but more pertinently following the peat failure in October 2003, during the construction of a wind farm at Derrybrien, County Galway, Ireland.

The geotechnical and peat stability assessment at the site included the following activities:

- (1) Desk study
- (2) Site reconnaissance including shear strength and peat depth measurements
- (3) Peat stability assessment of the peat slopes on site using a deterministic and qualitative approach
- (4) Peat contour depth plan is compiled based on the peat depth probes carried out across the site by FT (2019), HES (2019) and GPR survey data from Bord na Móna (2008 and 2015)
- (5) Factor of safety plan is compiled for the short term critical condition (undrained) for 579 no. FoS points analysed along the proposed infrastructure envelope on site
- (6) Construction buffer zone plan identifies areas with an elevated or higher construction risk where mitigation/control measures will need to be implemented during construction to minimise the potential risks and ensure they are kept within an acceptable range
- (7) A risk register was compiled to assess the potential design/construction risks at the infrastructure locations and determine adequate mitigation/control measures for each location to minimise the potential risks and ensure they are kept within an acceptable range, where necessary

- (8) Review of ground investigation carried out at the site by FT
- (9) Preliminary assessment of foundation type for turbines
- (10) Commentary of founding details for other infrastructure elements such as access roads, crane hardstands, substation compound platform, construction compound platform and met mast foundation

A flow diagram showing the general methodology for peat stability assessment is shown in Figure 2-1. The methodology illustrates the optimisation of the wind farm layout based on the findings from a site reconnaissance and subsequent feedback from the peat stability and risk assessment results.





2.4 Peat Failure Definition

Peat failure in this report refers to a significant mass movement of a body of peat that would have an adverse impact on proposed wind farm development and the surrounding environment. Peat failure excludes localised movement of peat that would occur (say) below an access road, creep movement or erosion type events.

The potential for peat failure at this site is examined with respect to wind farm construction and associated activity.

2.5 Main Approaches to Assessing Peat Stability

The main approaches for assessing peat stability for wind farm developments include the following:

- (a) Geomorphological
- (b) Qualitative (judgement)
- (c) Index/Probabilistic (probability)
- (d) Deterministic (factor of safety)

Approaches (a) to (c) listed above would be considered subjective and do not provide a definitive indication of stability; in addition, a high level of judgement/experience is required which makes it difficult to relate the findings to real conditions. FT apply a more objective approach, the deterministic approach (as discussed in section 2.5).

As part of FT's deterministic approach, a qualitative risk assessment is also carried out taking into account qualitative factors, which cannot necessarily be quantified, such as the presence of mechanically cut peat, quaking peat, bog pools, sub peat water flow, slope characteristics and numerous other factors. The qualitative factors used in the risk assessment are compiled based on FT's experience of assessments and construction in peat land sites and peat failures throughout Ireland and the UK. This approach follows the guidelines for geotechnical risk management as given in Clayton (2001), as referenced in the best practice for Peat Landslide Hazard and Risk Assessment Guide (PLHRAG, 2017), and takes into account the approach of MacCulloch (2005).

The risk assessment uses the results of the deterministic approach in combination with qualitative factors, which cannot be reasonably included in a stability calculation but nevertheless may affect the occurrence of peat instability to assess the risk of instability on a peat land site.

2.6 Peat Stability Assessment – Deterministic Approach

The peat stability assessment is carried out across a wide area of peatland to determine the stability of peat slopes and to identify areas of peatland that are suitable for development; this allows the layout of infrastructure on a particular wind farm site to be optimised. The assessment provides a numerical value (factor of safety) of the stability of individual parcels of peatland. The findings of the assessment discriminate between areas of stable and unstable peat, and areas of marginal stability where restrictions may apply. This allows for the identification of the most suitable locations for turbines, access roads and infrastructure.

A deterministic assessment requires geotechnical information and site characteristics which are obtained from desk study and site walkover, e.g. properties of peat/soil/rock, slope geometry, depth of peat, underlying strata, groundwater, etc. An adverse combination of the factors listed above could potentially result in instability. Using the information above a factor of safety is calculated for the stability of individual parcels of peatland on a site (as discussed in section 8).

The factor of safety is a measure of the stability of a particular slope. For any slope, the degree of stability depends on the balance of forces between the weight of the soil/peat working downslope (destabilising force) and the inherent strength of the peat/soil (shear resistance) to resist the downslope weight, see Figure 2-2.



Figure 2-2: Peat Slope Showing Balance of Forces to Maintain Stability

The factor of safety provides a direct measure of the degree of stability of a slope and is the ratio of the shear resistance over the downslope destabilising force. Provided the available shear resistance is greater than the downslope destabilising force then the factor of safety will be greater than 1.0 and the slope will remain stable. If the factor of safety is less than 1.0 the slope is unstable and liable to fail. The acceptable factor of safety is 1.3.

2.7 Applicability of the Factor of Safety (Deterministic) Approach for Peat Slopes

The factor of safety approach is a standard engineering approach in assessing slopes which is applied to many engineering materials, such as peat, soil, rock, etc.

The factor of safety approach is included in the Peat Landslide Hazard and Risk Assessments Best Practice Guide for Proposed Electricity Generation Developments (PLHRAG, 2017); see section 5.3.1 of the guide. This guide provides best practice methods to identify, mitigate and manage peat slide hazards and associated risks in respect of consent applications for electricity generation projects.

Furthermore, the best practice guide notes that the results from the factor of safety approach 'has provided the most informative results' with respect to analysing peat stability (section 5.3.1 of the guide).

The factor of safety approach in this report includes undrained (short-term stability) and drained (long-term stability) analyses. The undrained condition is the critical condition for the development. The purpose of the drained analysis is to identify the relative susceptibility of rainfall-induced failures at the site.

Notwithstanding the above, the stability analysis used by FT in this report also includes qualitative factors to determine the potential for peat stability i.e. the analysis used does not solely rely on the factor of safety approach.

The deterministic analysis is considered an acceptable engineering design approach. This concurs with the best practice guide referenced above.

2.8 Assessment of Intense Rainfall and Extreme Dry Events on the Peat Slopes

The deterministic approach carried out by FT examines intense rainfall and extreme dry events. The deterministic approach includes an undrained (short-term stability) and drained (long-term stability) analysis to assess the factor of safety for the peat slopes against a peat failure.

The drained loading condition applies in the long-term. This condition examines the effect of in particular, the change in groundwater level as a result of rainfall on the existing stability of the natural/existing peat slopes. For the drained analysis the level of the water table above the failure surface is required to calculate the factor of safety for the peat slope.

In order to represent varying water levels within the peat slopes, a sensitivity analysis is carried out which assesses varying water level in the peat slopes i.e. water levels ranging between 0 and 100% of the peat depth is conducted, where 0% equates to the peat being completely dry and 100% equates to the peat being fully saturated.

By carrying out such a sensitivity analysis with varying water level in the peat slopes, the effects of intense rainfall and extreme dry events are considered and analysed. The results of which are presented in Section 8 of this report.

3 DESK STUDY AND SITE RECONNAISSANCE

3.1 Desk Study

The main relevant sources of interest with respect to the site include:

- Geological plans
- Ordnance survey plans
- Literature review of peat failures
- Review of ground investigation data

The Geological Survey of Ireland (GSI, 2003) geological plans for the site were used to verify the bedrock conditions.

The ordnance surveys plans were reviewed to determine if any notable features or areas of particular interest (from a geotechnical point of view) are present on the site.

The desk study also included a review of both published literature and GSI online dataset viewer (GSI, 2019) on peat failures/landslides in the vicinity of the site.

3.2 Site Reconnaissance

As part of the assessment of potential peat failure at the proposed site, FT carried out a site reconnaissance. This comprised walkover inspections of the site with recording of salient geomorphological features with respect to the wind farm development and to provide peat thickness and preliminary assessment of peat strength.

The following salient geomorphological features were considered:

- Active, incipient or relict instability (where present) within the peat deposits
- Presence of shallow valley or drainage line
- Wet areas
- Any change in vegetation
- Peat depth
- Slope inclination and break in slope

The survey covered the proposed locations for the turbine bases and associated infrastructure.

The method adopted for carrying out the site reconnaissance relied on practitioners carrying out a visual assessment of the site supplemented with measurement of slope inclinations.

4 FINDINGS OF SITE RECONNAISSANCE

4.1 **Previous Failures**

The investigation works carried out at the study area have been used in conjunction with a desk study review to assess the susceptibility of the study area to peat failure.

There are no recorded peat failures within the Derrinlough wind farm site (GSI, 2019).

The nearest recorded peat failure is located 3km northeast of the study area. The failure occurred at Pollagh in 1954. The failure mechanism and type is not specified.

Another failure occurred some 5km south of the study area in an area called Lisheen in 1920. The failure occurred within peat and it is reported that an old road was swallowed in the bog.

Based on the Geological Survey of Ireland's dataset viewer (GSI, 2019) no other peat or non-peat failures occurred within an 8km radius of the site. A number of failures/landslides are noted within the Slieve Bloom Mountains to the southeast of the site where the upland terrain is significantly different to the relatively flat terrain present on site.

Based on a broad assessment of landslide susceptibility the site was classified by the GSI (2019) as 'low' to 'moderately low' susceptibility, which is expected given the relatively flat terrain present.

The presence, or otherwise, of relict peat failures or clustering of relict failures within an area is an indicator that particular site conditions exist that pre-dispose a site to failure or not as the case may be. Hence based on the historical data reviewed and the terrain and ground conditions present on site it can be concluded that site conditions in the area of the Derrinlough site have low potential of peat failure.

4.2 Ground Conditions along Grid Connection

A connection between the Derrinlough wind farm and the national electricity grid will be necessary to export electricity.

It is proposed that the Derrinlough wind farm will connect to the national grid via the substation proposed as part of the wind farm development located in Cortullagh or Grove townland in the northeast of the site. See Figure 4-1 for the general layout of the proposed substation and its associated grid connection.

The proposed grid connection construction methodology, including proposals for water crossings (where required) on the underground cabling routes is described in Chapter 4 of the EIAR.

Should the proposed substation be connected to the National Grid via underground cabling, it is proposed to excavate the trenches for the underground cable at a uniform level in peat or other overburden material. The trenches will be 600mm wide and 1200mm deep.

The cable trench route is envisaged to encounter peat, lacustrine soil and locally till derived from limestone rock.

No peat stability or geotechnical issues are envisaged as a result of the proposed grid connection works.

4.3 Findings of Site Reconnaissance

The site reconnaissance comprised walkover inspection of the site from 22nd August to 23rd August and the 10th and 11th September. Weather conditions for the inspection varied from dry to heavy showers.

Ground investigation in the form of trial pits were carried out by FT/AGEC and Bord na Móna during the following dates:

• 29th and 30th January 2019

- 12th to 16th August 2019
- 7th and 8th October 2019
- 6th November 2019

The walkovers and ground investigations were carried out by geotechnical engineers experienced in peat failure assessment. The findings from the site reconnaissance have been used to optimise the layout of the infrastructure on site.

The main findings of the site reconnaissance are as follows:

- (1) The site which is typically flat consists predominantly of bare locally re-vegetated cut-away peat and shallow peat. The site has been extensively harvested by Bord na Móna using mechanical cutting. Some localised vegetated areas are present across the site (Appendix A – Photos 1 to 4).
- (2) Peat depths recorded during the FT walkovers, HES site data, ground investigation (trial pits) and from the ground penetrating radar surveys carried out Bord na Móna within the proposed infrastructure footprint on site ranged from 0 to 4.7m with an average of 1.1m. It should be noted that the peat thickness within the proposed infrastructure footprint is generally less than 2m with localised deeper deposits of up to 4.7m. The deeper peat areas were generally avoided when optimising the wind farm layout for site.
- (3) The peat depths recorded at the turbine locations varied from 0 to 2.3m with an average depth of 1.3m.
- (4) The site is relatively flat with drainage channels running typically southeast to northwest. The site is split into two areas, Drinagh to the east of the N62 national road and Clongawny to the west.
- (5) There are a number of existing Bord na Móna rail lines that pass through the bog that are used to transport milled peat (Photo 5).
- (6) The access roads for the wind farm comprise, predominantly, the construction of new proposed access roads. The construction of new proposed access roads will be carried out using predominantly a floated construction technique.
- (7) With respect to the new proposed access roads, peat depths are typically less than 2m with localised depths of up to 4.7m recorded.
- (8) Slope angles at the turbine locations range from 1 to 4 degrees. These slope angle readings were obtained using a combination of readings taken during the site reconnaissance by FT using handheld equipment, such as the Silva Clino Master which has an accuracy of +/- 0.25 degrees and from contour survey plans for site.
- (9) The slope angle quoted typically reflects the slope within the footprint of each infrastructure location. The flat topography/nature of the terrain on site highlights the low risk of peat failure.
- (10) Localised areas of ponding water were recorded. This is not unexpected given the ground conditions and the flat terrain present on site (Photo 2).
- (11) No evidence of past failures or any significant signs of peat instability were noted on site.
- (12) The findings of the site reconnaissance are as follows:
 - (a) The site which is typically flat consists predominantly of bare locally re-vegetated cut-away peat and intact shallow peat.
 - (b) The peat depth within the proposed infrastructure footprint is typically less than 2m with localised deeper deposits of up to 4.7m.
 - (c) The flat topography/nature of the terrain on site highlights the low risk of peat failure.
 - (d) A construction buffer zone plan has been produced for the site (Figure 4-2). This Figure shows areas which have an elevated or higher construction risk due to the terrain and features encountered during the site reconnaissance i.e. presence of relatively deep peat.
 - (e) The results of the peat depth probing, shear strength testing of the peat and qualitative factors identified on site have been used in the stability and risk assessment; the findings of which are shown on the construction buffer zone plan and factor of safety plan for site (Figures 4-2 and 8-1).

Based on the findings from the site reconnaissance, the proposed development footprint for the site would be considered to have a low risk of peat instability.



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FIGURE 4-2 : CONSTRUCTION BUFFER ZONE

Date - 12.02.20

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Construction Buffer Zone Legend:

Areas which have an elevated or higher construction risk (areas with deep weak and occasionally quaking peat). Areas where additional control/mitigation measures are required.



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5 GROUND INVESTIGATION

Ground investigations were carried out at the Derrinlough site by FT/AGEC and Bord na Móna. Ground investigation in the form of trial pits were carried out during the following dates:

- 29th and 30th January 2019
- 12th to 16th August 2019
- 7th and 8th October 2019
- 6th November 2019

The ground investigations by FT/AGEC comprised 68 no. trial pits with some laboratory testing. The trial pits were carried out at the main infrastructure locations on site and to investigate the potential to develop borrow pits across the site.

The laboratory testing carried out included the following:

- Classification testing for overburden material
- Minimum and maximum density values for over-burden material

The trial pits logs, photographs and associated laboratory testing for each of the respective ground investigations carried out in January 2019 and August/October/November 2019 are included within Appendix E and F of this report.

The purpose of the ground investigations was to assess the ground conditions at the main infrastructure locations and across the site. A ground investigation location plan is included as Figure 5-1 in this report.

5.1 Summary of Ground Conditions

The ground conditions at the site can be typically categorised into the following deposits:

Peat – Typically described as spongy and firm black and brown fibrous to amorphous peat. The hand vanes carried out in the peat indicate undrained shear strengths in the range 20 to 120kPa, with an average value of about 65kPa. The relatively high strengths are as a result of the extensive drainage works which has taken place on site for the harvesting of the peat. Peat thicknesses from the trial pits ranged from 0 to in excess of 4.3m.

Lacustrine Soil – Grey very soft, soft and locally firm sandy silty clay/clayey Silt (Marl) was encountered. The marl is considered to be a lacustrine deposit. Lacustrine soil is typically underlying all peat deposits on site and varies in thickness from less than 1m to several metres.

Till – Typically described as firm brown and grey till - comprises sand, gravel and boulders within a silty clay matrix. The thickness of the layer is variable and was locally encountered on site.

Groundwater recordings in the trial pits varied from none to seepages and inflows between 1.2 and 3.9m bgl.

5.2 Summary of Laboratory Tests

Based on the results of the particle size distribution (PSD) tests, the descriptions on the final trial pit logs have been updated.

Atterberg limit tests carried out on the lacustrine soil classify the material as Clay of low plasticity.

The minimum and maximum densities determined for the Silt/Clay (till) sample ranged from $1.55Mg/m^3$ to $2.14Mg/m^3$.

5.3 Summary of Geotechnical Parameters

Table 5-1 contains characteristic geotechnical parameters for the main material types likely to be encountered on the Derrinlough wind farm site. Where direct measurement of parameters has not been carried out, established correlations with measured properties have been used to derive values. Characteristic values are defined as a cautious estimate of the value affecting the occurrence of limit state based on clause 2.4.5.2 from Eurocode 7.

Table 5-1: Summary of Geotechnical Parameters

	Unit	Geotechnical Parameters			
Material Type/Strata	Weight	Undrained Parameters	Drained I	Parameters	
	γ (kN/m³)	c _u (kPa)	φ' (°)	c' (kPa)	
Peat	11	6 ⁽³⁾	25	4	
Lacustrine Soil	18	20	26	0	
Till	19	75	30	0	

Notes

Note (1) The above parameters are indicative only and have been derived based on experience and from a review of the ground investigation carried out at the site.

Note (2) Where direct measurement of parameters has not been carried out, established correlations with measured properties have been used to derive values.

Note (3) A lower bound undrained shear strength, cu for the peat of 6kPa was selected. The lowest recorded value on the Derrinlough wind farm site was 20kPa hence a value of 6kPa is a conservative value.



6 SITE GROUND CONDITIONS

6.1 Soils and Subsoils

The bog at the Derrinlough site forms part of the Boora Group of Bogs which have essentially formed in poorly drained topographical depressions within the Midlands. Prior to the growth of the bog the area would have comprised water-logged and shallow lakes, which since the end of the last Ice Age have become silted hence the formation of a blanket peat area.

Based on the site walkover and the exposures present at the site the superficial deposits were typically described as plastic to firm black/brown fibrous and amorphous peat overlying lacustrine soil (marl) overlying till.

A review of the GSI subsoils maps indicate that the site is underlain by predominantly cutover raised peat with lacustrine soil and some localised till derived from limestone rock.

6.2 Bedrock

The underlying bedrock was described by the Geological Survey of Ireland (GSI, 2003) and shown on Sheet 15 (Geology of Galway - Offaly). In the area of the Derrinlough site, Sheet 15 shows 2 no. bedrock formations.

The dominant bedrock formation is Waulsortian Limestone, which is described as pale-grey, sparry, fossiliferous (bryozoan) poly-mud micritic limestones, often massive knoll forms, with crinoidal or pale cherty shaly interbeds, frequently dolomitised.

A localised bedrock formation located in the northeast of the site is the Ballysteen formation which is described as dark-grey bioclastic, wackestone, packstone and grainstone, shale-parted limestones increasingly muddy upwards.

There are two mapped faults located on the site, typically with a northwest to southeast trend. These mapped faults are shown in Figure 8-7 of Chapter 8 in the EIAR.

Two geological heritage features are noted within the proposed development. The features are noted as Mushroom Rocks, described as an isolated upstanding rock in a woodland track and a grass field.

7 PEAT DEPTHS, STRENGTH AND SLOPE AT PROPOSED INFRASTRUCTURE LOCATIONS

As part of the site walkover, peat depth, in-situ peat strength and slope angles were recorded at various locations across the site.

7.1 Peat Depth

Peat depth probes were carried out at/near to proposed turbine locations and access roads and other main infrastructure elements. At turbine locations typically up to 5 probes were carried out around the turbine location, where accessible, and an average peat depth was calculated. The peat depth probe information was supplemented by the ground penetrating radar survey carried out by Bord na Móna and was also used to estimate the peat depth on site.

7.2 Peat Strength

The strength testing was carried out in-situ using a Geonor H-60 Hand-Field Vane Tester. From FT's experience hand vanes give indicative results for in-situ strength of peat and would be considered best practice for the field assessment of peat strength. Peat strength testing was carried out at various locations across the site to give representative coverage of the full site.

7.3 Slope Angle

The slope angles at each of the main infrastructure locations were obtained using a combination of readings taken during the site reconnaissance by FT using handheld equipment, such as the Silva Clino Master which has an accuracy of +/-0.25 degrees and from contour survey plans for site.

The slope angle quoted typically reflects the slope within the footprint of each infrastructure location. It should be noted that slope angles derived from contour survey plans would be considered approximate, as such surveys are dependent on the density of survey data and do not always reflect local variations in ground topography. Slope angles recorded during the site reconnaissance by FT using handheld equipment and would generally be deemed more accurate and representative of local topography.

7.4 Summary of Findings

Based on the peat depth information for site, the peat varied in depth from 0 to 4.7m with an average of 1.1m. All peat depth probes carried out on site, ground investigation findings and the 2008 and 2015 GPR surveys carried out by Bord na Móna have been utilised to produce a peat depth contour plan for the site (Figure 4-1).

A summary of the peat depths at the proposed infrastructure locations is given in Table 7-1. The data presented in Table 7-1 is used in the peat stability assessment of the site; see Section 8 of this report.

Turbine	Easting (ITM)	Northing (ITM)	Peat Depth Range (m) ⁽¹⁾	Average Peat Depth (m)	Slope Angle (°) (2)
T1	607027	715375	0.2 - 0.5	0.35	1
T2	607171	714768	0.8 - 1.4	1.1	1.5
Т3	606666	713348	0.8 - 1.4	1.3	1
T4	605958	713100	1.1 - 1.8	1.5	2
T5	606961	712824	1.7 - 2.3	2.0	1
Т6	606452	714870	1.0 - 1.6	1.3	1.9
T7	605724	714916	2.0 - 2.3	2.2	1
Т8	605919	715618	0.7 - 1.8	1.4	1
Т9	605954	714337	1.2 - 1.7	1.4	1
T10	605094	714562	1.0 - 1.5	1.3	2
T11	605273	714023	0.8 - 1.8	1.1	3.1
T12	609661	715257	0.3 - 0.8	0.6	1
T13	609600	714503	0.2 - 0.8	0.45	2
T14	609982	714058	1.2 - 1.5	1.35	2
T15	610807	714384	0.5 – 0.7	0.6	2
T16	610313	714732	0.5 – 0.6	0.55	1
T17	610068	715972	0.15 - 0.8	0.5	3
T18	609958	716616	0.3 - 0.8	0.6	4.1
T19	609360	715724	0.2 - 1.2	0.9	1
T20	609307	716406	0.8 - 1.1	0.9	1
T21	609761	717087	1.2 - 1.6	1.4	2.9
Substation	610966	718547	0.3 - 1.1	0.9	1
Temporary Construction Compound 1	606260	714498	2.2 - 3.3	2.5	1
Temporary Construction Compound 2	607630	715110	0.3 - 1.3	0.6	1
Temporary Construction Compound 3	609113	715577	0.4 - 2.1	1.4	1
Temporary Construction Compound 4	610305	715713	0.4 - 1.5	1.0	3
Temporary Construction Compound 5	610858	718421	0.8 - 1.1	1.0	1
Met Mast 1	606638	712525	2.7 - 3.5	3.1	1

Table 7-1: Peat Depth and Slope Angle at Proposed Infrastructure Locations

Turbine	Easting (ITM)	Northing (ITM)	Peat Depth Range (m) ⁽¹⁾	Average Peat Depth (m)	Slope Angle (°) (2)
Met Mast 2	609210	714421	0.3 - 1.1	0.8	1

Note (1) Based on peat depth probe results from the site walkovers, ground investigation and from a ground penetrating radar (GPR) survey of site carried out by Bord na Móna in 2008 and 2015.

- Note (2) The slope angles at each of the main infrastructure locations were obtained using a combination of readings taken during the site reconnaissance by FT using handheld equipment, such as the Silva Clino Master (which has an accuracy of +/- 0.25 degrees) and from contour survey plans for site. The slope angle quoted typically reflects the slope within the footprint of each infrastructure location.
- Note (3) The data presented in the Table above is used in the peat stability assessment of the site; see Section 8 of this report.

In addition to probing, in-situ shear vane testing was carried out as part of the ground investigation. Strength testing was carried out at selected locations across the site to provide representative coverage of indicative peat strengths. The results of the vane testing are presented in Figure 7-1.

The hand vane results indicate undrained shear strengths in the range 20 to 120kPa, with an average value of about 65kPa. The strengths recorded would be typical of well drained peat as is present on the Derrinlough site.

Peat strength at sites of known peat failures (assuming undrained loading failure) are generally very low, for example the undrained shear strength at the Derrybrien failure (AGEC, 2004) as derived from essentially back-analysis, though some testing was carried out, was estimated at 2.5kPa. The recorded undrained strength is significantly greater than the lower bound values for Derrybrien indicating that there is no close correlation to the peat conditions at the Derrybrien site and that there is significantly less likelihood of failure on the Derrinlough site.





8 PEAT STABILITY ASSESSMENT

The peat stability assessment analyses the stability of the existing peat slopes for individual parcels across the site including at the turbine locations and along the proposed access roads. The assessment also analyses the stability of the existing peat slopes with a surcharge loading of 10kPa, equivalent to placing 1m of stockpiled peat on the surface of the peat slope.

8.1 Methodology for Peat Stability Assessment

Stability of a peat slope is dependent on several factors working in combination. The main factors that influence peat stability are slope angle, shear strength of peat, depth of peat, pore water pressure and loading conditions.

An adverse combination of factors could potentially result in peat sliding. An adverse condition of one of the above-mentioned factors alone is unlikely to result in peat failure. The infinite slope model (Skempton and DeLory, 1957) is used to combine these factors to determine a factor of safety for peat sliding. This model is based on a translational slide, which is a reasonable representation of the dominant mode of movement for peat failures.

To assess the factor of safety for a peat slide, an undrained (short-term stability) and drained (long-term stability) analysis has been undertaken to determine the stability of the peat slopes on site.

- 1. The undrained loading condition applies in the short-term during construction and until construction induced pore water pressures dissipate.
- 2. The drained loading condition applies in the long-term. The condition examines the effect of in particular, the change in groundwater level as a result of rainfall on the existing stability of the existing peat slopes.

Undrained shear strength values (cu) for peat are used for the total stress analysis. Based on the findings of the Derrybrien failure, undrained loading during construction was found to be the critical failure mechanism.

A drained analysis requires effective cohesion (c') and effective friction angle (\emptyset ') values for the calculations. These values can be difficult to obtain because of disturbance experienced when sampling peat and the difficulties in interpreting test results due to the excessive strain induced within the peat. To determine suitable drained strength values a review of published information on peat was carried out.

Table 8-1 shows a summary of the published information on peat together with drained strength values.

Table 8-1: List of Effective Cohesion and Friction Angle Values

Reference	Cohesion, c' (kPa)	Friction Angle, ø' (degs)	Testing Apparatus/ Comments
Hanrahan et al (1967)	5 to 7	36 to 43	From triaxial apparatus
Rowe and Mylleville (1996)	2.5	28	From simple shear apparatus
Landva (1980)	2 to 4	27.1 to 32.5	Mainly ring shear apparatus for normal stress greater than 13kPa
	5 to 6	-	At zero normal stress
Carling (1986)	6.5	0	-
	0	38	From ring shear and shear box apparatus. Results are not considered representative.
(1998)	0.61	31	From direct simple shear (DSS) apparatus. Result considered too low therefore DSS not considered appropriate
Rowe, Maclean and	1.1	26	From simple shear apparatus
Soderman (1984)	3	27	From DSS apparatus
McGreever and	6	38	From triaxial apparatus using soil with 20% organic content
Farrell (1988)	6	31	From shear box apparatus using soil with 20% organic content
Hungr and Evans (1985)	3.3	-	Back-analysed from failure
Dykes and Kirk (2006)	3.2	30.4	Test within acrotelm
Dykes and Kirk (2006)	4	28.8	Test within catotelm
Warburton et al (2003)	5	23.9	Test in basal peat
Warburton et al (2003)	8.74	21.6	Test using fibrous peat
Hendry et al (2012)	0	31	Remoulded test specimen
Komatsu et al (2011)	8	34	Remoulded test specimen
Zwanenburg et al (2012)	2.3	32.3	From DSS apparatus
Den Haan and Grognet (2014)	-	37.4	From large DSS apparatus
O'Kelly and Zhang (2013)	0	28.9 to 30.3	Tests carried out on reconstituted, undisturbed and blended peat samples

From Table 8-1 the values for c' ranged from 1.1 to 8.74kPa and \emptyset' ranged from 21.6 to 43°. The average c' and \emptyset' values are 4.5kPa and 30° respectively. Based on the above, it was considered to adopt a conservative approach and to use design values below the averages.

For design the following general drained strength values have been used for the site:

c' = 4kPa $\phi' = 25$ degrees

8.2 Analysis to Determine Factor of Safety (Deterministic Approach)

The purpose of the analysis was to determine the Factor of Safety (FoS) of the peat slopes using infinite slope analysis. The analysis was carried out at the turbine locations, along the proposed access roads and at various locations across the site.

The FoS provides a direct measure of the degree of stability of the slope. A FoS of less than unity indicates that a slope is unstable, a FoS of greater than unity indicates a stable slope.

The acceptable safe range for FoS typically ranges from 1.3 to 1.4. The previous code of practice for earthworks BS 6031:1981 (BSI, 1981), provided advice on design of earthworks slopes. It stated that for a first time failure with a good standard of site investigation the design FoS should be greater than 1.3.

As a general guide the FoS limits for peat slopes in this report are summarised in Table 8-2.

Factor of Safety (FoS)	Degree of Stability	
Less than 1.0	Unstable (red)	
Between 1.0 and 1.3	Marginally stable (yellow)	
1.3 or greater	Acceptable (green)	

Table 8-2: Factor of Safety Limits for Slopes

Eurocode 7 (EC7) (IS EN 1997-1:2005) now serves as the reference document and the basis for design geotechnical engineering works. The design philosophy used in EC7 applies partial factors to soil parameters, actions and resistances. Unlike the traditional approach, EC7 does not provide a direct measure of stability, since global Factors of Safety are not used.

As such, and in order to provide a direct measure of the level of safety on a site, EC7 partial factors have not been used in this stability assessment. The results are given in terms of FoS.

A lower bound undrained shear strength, c_u for the peat of 6kPa was selected for the assessment. The lowest recorded value on the Derrinlough wind farm site was 20kPa. It should be noted that a c_u of 6kPa for the peat is considered a conservative value for the analysis and is not representative of all peat present across the site. In reality the peat has a significantly higher undrained strength as a result of the extensive drainage and extraction works which have been carried out on site.

The formula used to determine the factor of safety for the undrained condition in the peat (Bromhead, 1986) is as follows:

 $F = \frac{c_u}{\gamma z \sin \alpha \cos \alpha}$

Where:

- F = Factor of Safety
- cu = Undrained strength
- γ = Bulk unit weight of material
- z = Depth to failure plane assumed as depth of peat
- a = Slope angle

The formula used to determine the factor of safety for the drained condition in the peat (Bromhead, 1986) is as follows:

$$F = \frac{c' + (\gamma_z - \gamma_w h_w) \cos^2 \alpha \tan \phi'}{\gamma_z \sin \alpha \cos \alpha}$$

Where:

- *F* = Factor of Safety
- c' = Effective cohesion
- γ = Bulk unit weight of material
- z = Depth to failure plane assumed as depth of peat
- γw = Unit weight of water
- hw = Height of water table above failure plane
- a = Slope angle
- $\phi' =$ Effective friction angle

For the drained analysis the level of the water table above the failure surface is required to calculate the factor of safety for the slope. Since the water level in blanket peat can be variable and can be recharged by rainfall, it is not feasible to establish its precise location throughout the site. Therefore a sensitivity analysis using water level ranging between 0 and 100% of the peat depth was conducted, where 0% equates to the peat been completely dry and 100% equates to the peat been fully saturated.

The following general assumptions were used in the analysis of peat slopes at each location:

- (1) Peat depths are based on the maximum peat depth recorded at each location from the walkover surveys, ground investigation or from the GPR surveys carried out by Bord na Móna.
- (2) The slope angles used in the peat stability assessment were obtained using a combination of readings taken during the site reconnaissance by FT using handheld equipment and from contour survey plans for site. It should be noted that slope angles derived from contour survey plans would be considered approximate, as such surveys are dependent on the density of survey data and do not always reflect local variations in ground topography. In general the slope angles derived from the contour survey plans appear to have over-estimated the steepness of the slopes on site.
- (3) Slope angle on base of sliding assumed to be parallel to ground surface.
- (4) A lower bound undrained shear strength, cu for the peat of 6kPa was selected for the assessment. The lowest recorded value on the Derrinlough wind farm site was 20kPa. It should be noted that a cu of 6kPa for the peat is considered a conservative value for the analysis and is not representative of all peat present across the site. In reality the peat has a significantly higher undrained strength as a result of the extensive drainage and extraction works which have been carried out on site.

For the stability analysis two load conditions were examined, namely

Condition (1): no surcharge loading Condition (2): surcharge of 10 kPa, equivalent to 1 m of stockpiled peat assumed as a worst case.

8.3 Results of Analysis

8.3.1 Undrained Analysis for the Peat

The results of the undrained analysis for the existing peat slopes are presented in Appendix C and the results of the undrained analysis for the most critical load case (load condition 2) are shown on Figure 8-1. The undrained analysis for load condition 2 is considered the most critical load case as most peat failures occur in the short term upon loading of the peat surface. The results from the main infrastructure locations are summarised in Table 8-3.

The calculated FoS for load condition (1) is in excess of 1.30 for each of the locations (579 no. locations) analysed with a range of FoS of 1.62 to in excess of 10, indicating a low risk of peat instability.

The calculated FoS for load condition (2) is in excess of 1.30 for each of the locations (579 no. locations) except at 2 no. locations where FoS's of 1.21 and 1.26 were calculated. It should be noted that the locations where the marginally low FoS's were calculated, the slope angles were based on contour survey plans for site which give approximate values. Based on site data recorded during the walkover, it is likely that the slope angles derived from the contour survey plans overestimated the slope angle at these locations. The 2 no. marginally low FoS's are located alongside the proposed access road between turbines T12 and T13. The proposed works at these locations entails the construction of a floated section of access road i.e. no excavation works are proposed at these locations. Peat instability at these locations is not envisaged to be an issue.

The calculated FoS for load condition (2) for the remaining 577 no. locations were in excess of 1.30, indicating a low risk of peat instability.

Turbine	Easting Northin	Northing	Factor of Safety for Load Condition	
No./ Waypoint			Condition (1)	Condition (2)
T1	607027	715375	68.77	22.92
T2	607171	714768	16.38	9.55
Т3	606666	713348	24.56	14.33
T4	605958	713100	9.56	6.14
T5	606961	712824	14.95	10.42
Т6	606452	714870	11.32	6.96
T7	605724	714916	14.95	10.42
Т8	605919	715618	19.10	12.28
Т9	605954	714337	20.23	12.73
T10	605094	714562	11.47	6.88
T11	605273	714023	6.17	3.97
T12	609661	715257	42.98	19.10
T13	609600	714503	21.50	9.56
T14	609982	714058	11.47	6.88
T15	610807	714384	24.58	10.12
T16	610313	714732	57.31	21.49
T17	610068	715972	14.35	6.38
T18	609958	716616	10.52	4.67
T19	609360	715724	28.65	15.63

Table 8-3: Factor of Safety Results (undrained condition)

Turbine	Easting Northing		Factor of Safety for Load Condition	
NO./ Waypoint			Condition (1)	Condition (2)
T20	609307	716406	31.26	16.37
T21	609761	717087	7.42	4.57
Substation	610966	718547	31.26	16.37
Temporary Construction Compound 1	606260	714498	10.42	8.00
Temporary Construction Compound 2	607630	715110	26.45	14.95
Temporary Construction Compound 3	609113	715577	16.37	11.09
Temporary Construction Compound 4	610305	715713	7.65	4.59
Temporary Construction Compound 5	610858	718421	31.26	16.37
Met Mast 1	606638	712525	9.82	7.64
Met Mast 2	609210	714421	31.26	16.37



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8.3.2 Drained Analysis for the Peat

The results of the drained analysis for the peat are presented in Appendix C. The results from the main infrastructure locations are summarised in Table 8-4. As stated previously, the drained loading condition examines the effect of in particular, rainfall on the stability of the existing peat slopes.

The calculated FoS for load condition (1) is in excess of 1.30 for each of the locations (579 no. locations) analysed with a range of FoS of 1.30 to in excess of 10 except for 3 no. locations where FoS's of between 1.08 and 1.29 were calculated. It should be noted that the locations where the marginally low FoS's were calculated, the slope angles were based on contour survey plans for site which give approximate values. Based on site data recorded during the walkover, it is likely that the slope angles derived from the contour survey plans overestimated the slope angle at these locations. The 3 no. marginally low FoS's are located alongside the proposed access road between turbines T12 and T13 and one location along the proposed access road at turbine T4. The proposed works at these locations entails the construction of a floated section of access road i.e. no excavation works are proposed at these locations. Peat instability at these locations is not envisaged to be an issue.

The calculated FoS for load condition (2) for the remaining 576 no. locations were in excess of 1.30, indicating a low risk of peat instability.

The calculated FoS for load condition (2) is in excess of 1.30 for each of the locations (579 no. locations) analysed with a range of FoS of 1.71 to in excess of 10, indicating a low risk of peat instability.

Turbine No./Waypoint	Easting	Northing	Factor of Safety for Load Condition	
			Condition (1)	Condition (2)
T1	607027	715375	45.85	33.09
T2	607171	714768	10.92	13.79
Т3	606666	713348	16.37	20.68
T4	605958	713100	6.37	8.86
Т5	606961	712824	9.97	15.04
Т6	606452	714870	7.54	10.05
Т7	605724	714916	9.97	15.04
Т8	605919	715618	12.73	17.73
Т9	605954	714337	13.48	18.38
T10	605094	714562	7.65	9.93
T11	605273	714023	4.12	5.72
T12	609661	715257	28.65	27.58
T13	609600	714503	14.34	13.79
T14	609982	714058	7.65	9.93
T15	610807	714384	16.38	14.60
T16	610313	714732	38.20	31.02
T17	610068	715972	9.57	9.20
T18	609958	716616	7.01	6.73

Table 8-4: Factor of Safety Results (drained condition)

Turbine No./Waypoint	Easting	Northing	Factor of Safety for Load Condition	
			Condition (1)	Condition (2)
T19	609360	715724	19.10	22.56
T20	609307	716406	20.84	23.64
T21	609761	717087	4.95	6.59
Substation	610966	718547	20.84	23.64
Temporary Construction Compound 1	606260	714498	6.95	11.54
Temporary Construction Compound 2	607630	715110	17.63	21.58
Temporary Construction Compound 3	609113	715577	10.92	16.01
Temporary Construction Compound 4	610305	715713	5.10	6.62
Temporary Construction Compound 5	610858	718421	20.84	23.64
Met Mast 1	606638	712525	6.55	11.03
Met Mast 2	609210	714421	20.84	23.64

9 PEAT STABILITY RISK ASSESSMENT

A peat stability risk assessment was carried out for the main infrastructure elements at the wind farm. This approach takes into account guidelines for geotechnical/peat stability risk assessments as given in PLHRA (2017) and MacCulloch (2005).

The risk assessment uses the results of the stability analysis (deterministic approach) in combination with qualitative factors, which cannot be reasonably included in a stability calculation but nevertheless may affect the occurrence of peat instability, to assess the risk for each infrastructure element.

For each of the main infrastructure elements, a risk rating (product of probability and impact) is calculated and rated as shown in Table 9-1. Where a subsection is rated 'Medium' or 'High', control measures are required to reduce the risk to at least a 'Low' risk rating. Where a subsection is rated 'Low' or 'Negligible', only routine control measures are required.

Table 9-1: Risk Rating Legend

17 to 25	High: avoid works in area or significant control measures required		
11 to 16	Medium: notable control measures required		
5 to 10	Low: only routine control measures required		
1 to 4	Negligible: none or only routine control measures required		

A full methodology for the peat stability risk assessment is given in Appendix D.

9.1 Summary of Risk Assessment Results

The results of the peat stability risk assessment for potential peat failure at the main infrastructure elements is presented as a Peat Stability Risk Registers in Appendix B and summarised in Table 9-2.

The risk rating for each infrastructure element at the Derrinlough wind farm is designated negligible following some mitigation/control measures being implemented. Sections of access roads to the nearest infrastructure element should be subject to the same mitigation/control measures that apply to the nearest infrastructure element.

Details of the required mitigation/control measures can be found in the Peat Stability Risk Register for each infrastructure element (Appendix B).
Infrastructure Pre-Control Measure Implementati on Risk Rating Category		Notable Control Measures Required	Post-Control Measure Implementati on Risk Rating	Post-Control Measure Implementatio n Risk Rating Category	
Turbine T1	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T2	Negligible	1 to 4	1 to 4 No Negligible		1 to 4
Turbine T3	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T4	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T5	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T6	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T7	Negligible	1 to 4	Yes	Negligible	1 to 4
Turbine T8	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T9	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T10	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T11	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T12	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T13	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T14	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T15	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T16	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T17	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T18	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T19	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T20	Negligible	1 to 4	No	Negligible	1 to 4
Turbine T21	Negligible	1 to 4	No	Negligible	1 to 4
Substation	Negligible	1 to 4	No	Negligible	1 to 4
Temporary Construction Compound 1	Negligible	1 to 4	Yes	Negligible	1 to 4
Temporary Construction Compound 2	Negligible	1 to 4	No	Negligible	1 to 4
Temporary Construction Compound 3	Negligible	1 to 4	Yes	Negligible	1 to 4
Temporary Construction Compound 4	Negligible	1 to 4	No	Negligible	1 to 4
Temporary Construction Compound 5	Negligible	1 to 4	No	Negligible	1 to 4
Met Mast 1	Negligible	1 to 4	Yes	Negligible	1 to 4

Table 9-2: Summary of Peat Stability Risk Register

McCarthy Keville O'Sullivan Geotechnical and Peat Stability Assessment Derrinlough Wind Farm

Met Mast 2 Negligible 1 to 4 No Negligible 1 to 4	Met Mast 2	Negligible	1 to 4	No	Negligible	1 to 4
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10 INDICATIVE FOUNDATION TYPE AND FOUNDING DEPTH FOR TURBINES

Based on a review of the ground investigation information for site, a preliminary assessment of the likely foundation type and founding depths for each turbine location was carried out, where possible. A summary of this assessment is provided in Table 10-1.

Table 10-1: Summary of Indicative Turbine Foundation Type and Founding Depths

Turbine No.	Turbine Foundation Type	Relevant GI	Indicative founding depth (m bgl)	Comment
T1	Piled foundation	TP/T1	-	The site investigation works carried out indicate that a piled foundation will be required.
Т2	Piled foundation	TP/T2	-	The site investigation works carried out indicate that a piled foundation will be required.
Т3	Piled foundation	TP/T3	-	The site investigation works carried out indicate that a piled foundation will be required.
T4	Piled foundation	TP/T4	-	The site investigation works carried out indicate that a piled foundation will be required.
Т5	Piled foundation	TP/T5	-	The site investigation works carried out indicate that a piled foundation will be required.
T6	Piled foundation	TP/T6	-	The site investigation works carried out indicate that a piled foundation will be required.
Т7	Piled foundation	TP/T7	-	The site investigation works carried out indicate that a piled foundation will be required.
Т8	Piled foundation	TP/T8	-	The site investigation works carried out indicate that a piled foundation will be required.
Т9	Piled foundation	TP/T9	-	The site investigation works carried out indicate that a piled foundation will be required.
T10	Piled foundation	TP/T10	-	The site investigation works carried out in the area indicate that a piled foundation will be required.
T11	Piled foundation	TP/T11	-	The site investigation works carried out indicate that a piled foundation will be required.
T12	Piled foundation	TP/T12	-	The site investigation works carried out indicate that a piled foundation will be required.
T13	Piled foundation	TP/T13	-	The site investigation works carried out indicate that a piled foundation will be required.

Turbine No.	Turbine Foundation Type	Relevant GI	Indicative founding depth (m bgl)	Comment
T14	Piled foundation	TP/T14	-	The site investigation works carried out indicate that a piled foundation will be required.
T15	Piled foundation	TP/T15	-	The site investigation works carried out indicate that a piled foundation will be required.
T16	Piled foundation	TP/T16	-	The site investigation works carried out indicate that a piled foundation will be required.
T17	Piled foundation	TP/T17	-	The site investigation works carried out indicate that a piled foundation will be required.
T18	Piled foundation	TP/T18	-	The site investigation works carried out indicate that a piled foundation will be required.
T19	Piled foundation	TP/T19	-	The site investigation works carried out indicate that a piled foundation will be required.
T20	Piled foundation	TP/T20	-	The site investigation works carried out indicate that a piled foundation will be required.
T21	Piled foundation	TP/T21	-	The site investigation works carried out indicate that a piled foundation will be required.

It should be noted that further ground investigation will be carried out prior to construction at each turbine location in the form of a borehole with in-situ SPT testing at 1m intervals in the overburden and follow-on rotary core through bedrock to confirm the foundation types and founding stratums assumed in Table 10-1. It is likely that following the completion of further ground investigation prior to construction that a number of the turbine bases will be deemed suitable for gravity type foundations.

For gravity type turbine foundations, where the depth of excavation exceeds the required founding depth for the proposed turbine base, up-fill material consisting of granular fill (6N) shall be used to backfill the excavation to the required founding depth.

For the piled turbine foundations, a typical piling type and configuration could be up to 50 no. 300mm square concrete driven piles. A similar type pile and configuration was used for the turbine foundations on the nearby Mount Lucas wind farm.

11 FOUNDING DETAILS FOR OTHER INFRASTRUCTURE ELEMENTS

11.1 Access Roads

Floating access roads are the predominant road construction type proposed for the site which given the ground conditions and type of terrain present is deemed an appropriate construction approach.

The total length of new proposed access road to be constructed on site is 28.5km (see Figure 1-1 of the Peat and Spoil Management Plan).

The typical make-up of the founded access roads is a minimum stone thickness of 1200mm. The requirement for a layer of geotextile and geogrid and the necessary stone thickness will be determined following confirmatory site investigation works prior to the construction of the proposed development.

See the Peat and Spoil Management Plan for Derrinlough wind farm for further details on the proposed access roads on site.

11.2 Crane Hardstands

The crane hardstands will be constructed using the founded technique (i.e. non-floated technique).

Crane hardstands are generally constructed using compacted Class 1/6F material or granular fill in accordance with turbine manufacturer requirements on a suitable sub-formation to achieve the required bearing resistance. The hardstands will be designed for the most critical loading combinations from the crane.

The hardstands will require to be founded on material underlying the peat deposits. The founding levels for the hardstands will be variable across the site and will be determined following confirmatory site investigation works prior to the construction of the proposed development.

The typical make-up of the hardstands may include up to 1200mm of granular stone fill with possibly a layer of geotextile and/or geogrid.

11.3 Substation Foundations and Platforms

The substation platforms will be constructed using the founded technique (i.e. non-floated technique). The substation foundations may comprise strip/raft foundations under the main footprint of the building with possibly a basement/pit for cable connections.

Substation platforms are generally constructed using compacted Class 1/6F material or granular fill in accordance with Eirgrid/ESB network requirements on a suitable sub-formation to achieve the required bearing resistance.

The substation platforms will require to be founded on material underlying the peat deposits.

Given the ground conditions present at the proposed substations, it is envisaged that the foundations will require to be founded on till. The peat and lacustrine soils are not likely to be suitable founding stratums for the substation foundations.

Typical founding depth for substation platform likely to be 0.5 to 2.0m.

The typical make-up of the substation platform may include up to 1000mm of granular stone fill with possibly a layer of geotextile and/or geogrid. At the underside of the substation foundations, a layer of structural up-fill (class 6N/6P) in accordance with Eirgrid requirements will likely be required.

11.4 Temporary Construction Compound Platforms

The temporary construction compound platforms will be constructed using the founded technique (i.e. non-floated technique).

The platforms are generally constructed using compacted Class 1/6F material or granular fill in accordance with turbine manufacturer requirements on a suitable sub-formation to achieve the required bearing resistance.

The platforms will require to be founded on material underlying the peat deposits.

Typical founding depth for temporary construction compound platforms will require excavations from 1m to 3.5m bgl.

The typical make-up of the construction compound platform may include up to 1000mm of granular stone fill with possibly a layer of geotextile and/or geogrid.

11.5 Met Mast Foundations

The met mast foundations will likely comprise gravity type foundation and a piled foundation.

Based on the ground conditions present at proposed met mast 1, it is envisaged that the foundation will require a piled foundation. Given the thickness of peat and lacustrine soils present at this location a gravity type foundation is not likely to be a suitable solution. This will be determined following confirmatory site investigation works prior to the construction of the proposed development.

Based on the ground conditions present at proposed met mast 2, it is envisaged that the gravity type foundation will require to be founded on till. The peat and lacustrine soils are not likely to be suitable founding stratums for the met mast foundation. At the underside of the met mast foundation, a layer of structural up-fill (class 6N/6P) or granular fill in accordance with met mast supplier requirements will likely be required. This will be determined following confirmatory site investigation works prior to the construction of the proposed development.

11.6 Permanent Underpasses

Two new permanent underpasses are proposed as part of the proposed development.

The first underpass will traverse beneath the N62, immediately north of Derrinlough Briquette Factory. This underpass will provide amenity connectivity between Clongawny and Drinagh Bogs and will also be used during the operational phase for wind farm maintenance.

A second underpass is proposed in Clongawny bog beneath an existing Bord na Móna railway line. This underpass will also be used for amenity purposes and for wind farm maintenance during the operational phase.

The underpasses will take the form of precast concrete box culverts and will be founded on a competent stratum at depth. Given the ground conditions present across the site, the culvert foundations may need to be piled. This will be determined following confirmatory site investigation works prior to the construction of the proposed development.

At the underside of the culvert foundations, a layer of structural up-fill (class 6N/6P) in accordance with Transport Infrastructure Ireland (TII) requirements will be required.

12 SUMMARY AND RECOMMENDATIONS

12.1 Summary

The following summary is given.

FT was engaged by McCarthy Keville O'Sullivan on behalf of Bord na Móna Powergen Ltd to undertake a geotechnical and peat stability assessment of the proposed wind farm site.

The findings of the peat assessment showed that the site has an acceptable margin of safety and is suitable for the proposed wind farm development. The findings include recommendations and control measures for construction work in peat lands to ensure that all works adhere to an acceptable standard of safety.

The site which is typically flat consists predominantly of bare locally re-vegetated cut-away peat and intact shallow peat. The site has been extensively harvested and drained by Bord na Móna.

Peat thicknesses recorded during the FT walkover, ground investigation and from the ground penetrating radar surveys carried out by Bord na Móna within the proposed infrastructure footprint on site ranged from 0 to 4.7m with an average of 1.1m. It should be noted that the peat thickness within the proposed infrastructure footprint is generally less than 2m with localised deeper deposits of up to 4.7m. The deeper peat areas were generally avoided when optimising the wind farm layout for site.

Slope inclinations at the main infrastructure locations range from 0 to 4 degrees. The flat topography/nature of the terrain on site highlights the low risk of peat failure.

An analysis of peat sliding was carried out at the main infrastructure locations across site for both the undrained and drained conditions. The purpose of the analysis was to determine the Factor of Safety (FoS) of the peat slopes. The FoS provides a direct measure of the degree of stability of a peat slope. A FoS of less than 1.0 indicates that a slope is unstable; a FoS of greater than 1.0 indicates a stable slope. An acceptable FoS for slopes is generally taken as a minimum of 1.3.

From the stability analyses for both the undrained and drained conditions, which analysed the turbine locations and other proposed infrastructure locations, the calculated values were above the minimum acceptable FoS of 1.3 at 576 no. of the 579 no. locations. 3 no. locations were calculated with FoS's of between 1.08 and 1.29. The 3 no. marginally low FoS's are located alongside proposed access roads between turbines T12 and T13 and one location along the proposed access road close to turbine T4. The proposed works at these locations entails the construction of a floated section of access road i.e. no excavation works are proposed at these locations. Peat instability at these locations is not envisaged to be an issue.

As presented above for both the undrained and drained conditions, 100% of calculated FoS's are above 1.0 and 99.5% of calculated FoS's are above 1.3 which highlights the low risk of peat failure on site.

The peat stability risk assessment at each infrastructure location identified a number of mitigation/control measures to reduce the potential risk of peat failure. Sections of access roads should be subject to the same mitigation/control measures that apply to the nearest infrastructure element. See Appendix B for details of the required mitigation/control measures for each infrastructure element.

In summary the findings of the peat assessment showed that the proposed Derrinlough wind farm site has an acceptable margin of safety, is suitable for the proposed wind farm development and is considered to be at **low** risk of peat failure. The findings include recommendations and control measures for construction work in peat lands to ensure that all works adhere to an acceptable standard of safety.

12.2 Recommendations

The following general recommendations are given.

Notwithstanding that the site has an acceptable margin of safety a number of mitigation/control measures are given to ensure that all works adhere to an acceptable standard of safety for work in peatlands. Mitigation/control measures identified for each of the infrastructure elements in the risk assessment should be taken into account and implemented throughout design and construction works (Appendix B).

The proposed construction method for the new proposed access roads at the wind farm is a floated type construction.

Figure 4-2 shows areas which have an elevated or higher construction risk due to the terrain and features encountered during the site reconnaissance i.e. presence of relatively deep peat. Figure 8-1 shows the results of the factor of safety (FoS) analysis for the peat slopes on site for the most critical load condition.

Recommendations and guidelines given in FT's report 'Peat and Spoil Management Plan - Derrinlough Wind Farm, County Offaly' (FT 2019) should be taken into consideration during the design and construction stage of the wind farm development.

To minimise the risk of construction activity causing potential peat instability it is recommended that the Construction Method Statements (CMSs) for the project take into account, but not be limited, to the recommendations above. This will ensure that best practice guidance regarding the management of peat stability will be inherent in the construction phase.

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Appendix A

Photos from Site Visit











Photo 1 Overview of site conditions (close to turbine T5 looking in a north-eastern direction)



Photo 2 Overview of site conditions (close to temporary construction compound 2 looking in a southwestern direction)



Photo 3 Overview of site conditions (close to turbine T13 looking in a south-western direction)



Photo 4 Overview of site conditions (close to turbine T11 looking in a north-western direction)



Photo 5 Example of Bord na Móna existing rail lines on site (close to turbine T17 looking in a north-eastern direction)

Appendix B

Peat Stability Risk Registers









Location:	Turbine	Turbine T1			
Grid Reference (Eastings, Northings):	607027 71	5375			
Distance to Watercourse (m)	> 150				
Min & Max Measured Peat Depth (m):	0.2 to 0.5	5			
Control Required:	No	No			

		Pre-	Control Mea	sure Imple	ementation			Pos	t-Control N	leasure Im	plementation		
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating		
1	FOS = 22.91 (u), 33.09 (d)	1	1	1	Negligible	No		1	1	1	Negligible		
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible		
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible		
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible		
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible		
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No				1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
10	Evidence of bog pools	0	1	0	Not Applicable	No]	0	1	0	Not Applicable		
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable		

	Control Measures to be Implemented Prior to/and During Construction for Turbine T1
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T2			
Grid Reference (Eastings, Northings):	607171 714768			
Distance to Watercourse (m)		> 150		
Min & Max Measured Peat Depth (m):		0.8 to 1.4		
Control Required:		No		

		Pre-Control Measure Implementation						Post-Control Measure Implementation					
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating		
1	FOS = 9.55 (u), 10.92 (d)	1	1	1	Negligible	No		1	1	1	Negligible		
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible		
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible		
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible		
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible		
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No				1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
10	Evidence of bog pools	0	1	0	Not Applicable	No]	0	1	0	Not Applicable		
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable		

	Control Measures to be Implemented Prior to/and During Construction for Turbine T2
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:		Turbine T3			
Grid Reference (Eastings, Northings):	60	606666 713348			
Distance to Watercourse (m)		> 150			
Min & Max Measured Peat Depth (m):		0.8 to 1.4			
Control Required:		No			

		Pre-	Control Mea	sure Imple	ementation			Pos	t-Control N	leasure Im	plementation		
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating		
1	FOS = 14.33 (u), 16.37 (d)	1	1	1	Negligible	No		1	1	1	Negligible		
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible		
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible		
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible		
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible		
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No				1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable		

	Control Measures to be Implemented Prior to/and During Construction for Turbine T3
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
III	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbi	ne T4	
Grid Reference (Eastings, Northings):	605958	713100	
Distance to Watercourse (m)	> 150		
Min & Max Measured Peat Depth (m):	1.1 to 1.8		
Control Required:	N	lo	

		Pre-	Control Mea	sure Imple	ementation			Pos	Post-Control Measure Implementat							
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating					
1	FOS = 6.14 (u), 6.37 (d)	1	1	1	Negligible	No		1	1	1	Negligible					
2	Evidence of sub peat water flow	1	1	1	Negligible	No							1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible					
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible					
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible					
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible					
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable					

	Control Measures to be Implemented Prior to/and During Construction for Turbine T4
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:		Turbi	ne T5
			1
Grid Reference (Eastings, Northings):	e	606961	712824
Distance to Watercourse (m)		> 1	50
Min & Max Measured Peat Depth (m):		1.7 t	o 2.3
Control Required:		N	o

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implemen			plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 10.42 (u), 9.97 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No	1	0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Relatively deep peat	3	1	3	Negligible	Yes		1	1	1	Negligible

	Control Measures to be Implemented Prior to/and During Construction for Turbine T5
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbi	ne T6	
Grid Reference (Eastings, Northings):	606452	714870	
Distance to Watercourse (m)	> 150		
Min & Max Measured Peat Depth (m):	1.0 t	o 1.6	
Control Required:	N	lo	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implemen			plementation					
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating					
1	FOS = 6.96 (u), 7.54 (d)	1	1	1	Negligible	No		1	1	1	Negligible					
2	Evidence of sub peat water flow	1	1	1	Negligible	No							1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible					
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible					
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible					
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible					
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable					
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable					

	Control Measures to be Implemented Prior to/and During Construction for Turbine T6
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T7
Grid Reference (Eastings, Northings):	605724 714916
Distance to Watercourse (m)	> 150
Min & Max Measured Peat Depth (m):	2.0 to 2.3
Control Required:	Yes

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implemen			plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 10.42 (u), 9.97 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No	1	0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Relatively deep peat	3	1	3	Negligible	Yes		1	1	1	Negligible

	Control Measures to be Implemented Prior to/and During Construction for Turbine T7
i	Due to relatively deep peat at this turbine location, additional construction measures such as the following may be required:
	- excavation side walls to be supported (eg. boulders, retaining wall units) or excavation face battered to shallow angle
	- temporary works designer may be required to provide excavation support design
	- daily detailed inspection of excavation faces
	- potential for greater water inflow into excavation requiring removal of water using pumping
	- increased exclusion zone around excavation to avoid accidental loading of crest of slope
ii	Maintain hydrology of area as far as possible;
iii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iv	Use of experienced geotechnical staff for site investigation;
v	Use of experienced contractors and trained operators to carry out the work;
vi	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vii	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T8		
Grid Reference (Eastings, Northings):	605919 715618		
Distance to Watercourse (m)	> 150		
Min & Max Measured Peat Depth (m):	0.7 to 1.8		
Control Required:	No		

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation					
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating		
1	FOS = 12.28 (u), 12.73 (d)	1	1	1	Negligible	No		1	1	1	Negligible		
2	Evidence of sub peat water flow	1	1	1	Negligible	No				1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible		
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No			0	1	0	Not Applicable	
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible		
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible		
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible		
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable		

	Control Measures to be Implemented Prior to/and During Construction for Turbine T8
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Tur	bine T9
Grid Poference (Eastings, Northings);	605054	714227
Distance to Watercourse (m)	605954	> 150
Min & Max Measured Peat Depth (m):	1.2	2 to 1.7
Control Required:		No

		Pre-	Control Measu	re Implem	entation			Post-Control Measure Implementation																							
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating																				
1	FOS = 12.73 (u), 13.48 (d)	1	1	1	Negligible	No		1	1	1	Negligible																				
2	Evidence of sub peat water flow	1	1	1	Negligible	No																						1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No																						1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No								0	1	0	Not Applicable														
5	Type of vegetation	2	1	2	Negligible	No			2	1	2	Negligible																			
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible																				
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No					1	1	1	Negligible																	
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable																				
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable																				
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable																				
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable																				

	Control Measures to be Implemented Prior to/and During Construction for Turbine T9
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T10		
Grid Reference (Eastings, Northings):	605094	714562	
Distance to Watercourse (m)	> 150		
Min & Max Measured Peat Depth (m):	1.0 t	o 1.5	
Control Required:	N	lo	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation														
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating											
1	FOS = 6.88 (u), 7.65 (d)	1	1	1	Negligible	No		1	1	1	Negligible											
2	Evidence of sub peat water flow	1	1	1	Negligible	No													1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible											
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable											
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible											
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible											
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible											
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable											
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable											
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable											
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable											

	Control Measures to be Implemented Prior to/and During Construction for Turbine T10
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:		Turbiı	1e T11
Grid Reference (Eastings, Northings):	605273	714023	
Distance to Watercourse (m)		>1	50
Min & Max Measured Peat Depth (m):		0.8 t	o 1.8
Control Required:		N	lo

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation													
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating										
1	FOS = 3.97 (u), 4.12 (d)	1	1	1	Negligible	No		1	1	1	Negligible										
2	Evidence of sub peat water flow	1	1	1	Negligible	No												1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible										
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable										
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible										
6	General slope characteristics upslope/downslope from infrastructure location	2	1	2	Negligible	No	See Below	2	1	2	Negligible										
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible										
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable										
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable										
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable										
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable										

	Control Measures to be Implemented Prior to/and During Construction for Turbine T11
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	-ocation: Turbine 1			
Grid Reference (Eastings, Northings):		609661	715257	
Distance to Watercourse (m)		> 150		
Min & Max Measured Peat Depth (m):		0.3 t	o 0.8	
Control Required:		N	lo	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation				
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	
1	FOS = 19.10 (u), 27.58 (d)	1	1	1	Negligible	No		1	1	1	Negligible	
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible	
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible	
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible	
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible	
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No			1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable	

	Control Measures to be Implemented Prior to/and During Construction for Turbine T12
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Tur	oine T13
Grid Reference (Eastings, Northings):	60960	0 714503
Distance to Watercourse (m)		> 150
Min & Max Measured Peat Depth (m):	0.:	2 to 0.8
Control Required:		No

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation				
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	
1	FOS = 9.56 (u), 13.79 (d)	1	1	1	Negligible	No		1	1	1	Negligible	
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible	
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible	
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible	
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible	
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No			1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable	

	Control Measures to be Implemented Prior to/and During Construction for Turbine T13
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:		Turbiı	ne T14	
Grid Reference (Eastings, Northings):		609982	714058	
Distance to Watercourse (m)	Γ	> 150		
Min & Max Measured Peat Depth (m):		1.2 t	o 1.5	
Control Required:		N	lo	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation			
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 6.88 (u), 7.65 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Turbine T14
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T15			
Grid Reference (Eastings, Northings):	610807	714384		
Distance to Watercourse (m)	> 150			
Min & Max Measured Peat Depth (m):	0.5 to 0.7			
Control Required:	No			

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation				
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	
1	FOS = 10.12 (u), 14.6 (d)	1	1	1	Negligible	No		1	1	1	Negligible	
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible	
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible	
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible	
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible	
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No			1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable	
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable	

	Control Measures to be Implemented Prior to/and During Construction for Turbine T15
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbi	ne T16
	-	-
Grid Reference (Eastings, Northings):	610313	714732
Distance to Watercourse (m)	>1	50
Min & Max Measured Peat Depth (m):	0.5 t	o 0.6
Control Required:	N	lo

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation			
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 21.49 (u), 31.02 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Turbine T16
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:		Turbiı	ne T17	
Grid Reference (Eastings, Northings):	610068 715972			
Distance to Watercourse (m)		> 150		
Min & Max Measured Peat Depth (m):		0.15	to 0.8	
Control Required:		N	io	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation									
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating						
1	FOS = 6.38 (u), 9.20 (d)	1	1	1	Negligible	No		1	1	1	Negligible						
2	Evidence of sub peat water flow	1	1	1	Negligible	No								1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible						
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible						
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible						
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible						
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable						

	Control Measures to be Implemented Prior to/and During Construction for Turbine T17
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T18			
Grid Reference (Eastings, Northings):	609958	716616		
Distance to Watercourse (m)	> 150			
Min & Max Measured Peat Depth (m):	0.3 to 0.8			
Control Required:	N	lo		

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation									
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating						
1	FOS = 4.67 (u), 6.73 (d)	1	1	1	Negligible	No		1	1	1	Negligible						
2	Evidence of sub peat water flow	1	1	1	Negligible	No								1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible						
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible						
6	General slope characteristics upslope/downslope from infrastructure location	2	1	2	Negligible	No	See Below	2	1	2	Negligible						
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible						
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable						

	Control Measures to be Implemented Prior to/and During Construction for Turbine T18
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	٦	Furbiı	ne T19	
Grid Reference (Eastings, Northings):	60	9360	715724	
Distance to Watercourse (m)		> 150		
Min & Max Measured Peat Depth (m):		0.2 t	o 1.2	
Control Required:		N	lo	

		Pre-	Control Mea	sure Imple	ementation			Post-Control Measure Implementation									
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating						
1	FOS = 15.63 (u), 19.10 (d)	1	1	1	Negligible	No		1	1	1	Negligible						
2	Evidence of sub peat water flow	1	1	1	Negligible	No								1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible						
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible						
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible						
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible						
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable						
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable						

	Control Measures to be Implemented Prior to/and During Construction for Turbine T19
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turbine T20			
Grid Reference (Eastings, Northings):	609307 716406			
Distance to Watercourse (m)	> 150			
Min & Max Measured Peat Depth (m):	0.8 to 1.1			
Control Required:	N	ю		

			Pre-Control Measure Implementation					Post-Control Measure Implementation			
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 16.37 (u), 20.84 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No	See Below	0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No		1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Turbine T20
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vi	Based on available ground investigation information, piled turbine foundation may be required.

Location:	Turb	Turbine T21			
Grid Reference (Eastings, Northings):	609761	717087			
Distance to Watercourse (m)	>	> 150			
Min & Max Measured Peat Depth (m):	1.2	1.2 to 1.6			
Control Required:		No			

		Pre-	Pre-Control Measure Implementation			Post-Control Measure Implementation					
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 4.57 (u), 4.95 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No	See Below	0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No		1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Turbine T21				
i	Maintain hydrology of area as far as possible;				
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;				
iii	Use of experienced geotechnical staff for site investigation;				
iv	Use of experienced contractors and trained operators to carry out the work;				
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;				
vi	Based on available ground investigation information, piled turbine foundation may be required.				
Location: Substation					
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Grid Reference (Eastings, Northings):	610966	718547			
Distance to Watercourse (m)	> 1	50			
Min & Max Measured Peat Depth (m):	0.3 t	o 1.1			
Control Required:	N	No			

		Pre-	Control Measu	re Impleme	entation			Pos	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 16.37 (u), 20.84 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Substation
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Temp. C	Temp. Const. Comp. 1					
Grid Reference (Eastings, Northings):	606260	714498					
Distance to Watercourse (m)	> '	> 150					
Min & Max Measured Peat Depth (m):	2.2 t	2.2 to 3.3					
Control Required:	Y	Yes					

		Pre-	Control Measu	ure Implem	entation			Post	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 8.00 (u), 6.95 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Relatively deep peat	3	1	3	Negligible	Yes		1	1	1	Negligible

	Control Measures to be Implemented Prior to/and During Construction for Temporary Construction Compound 1
i	Due to relatively deep peat at this temporary construction compound, additional construction measures such as the following may be required: - excavation side walls to be supported (eg. boulders, retaining wall units) or excavation face battered to shallow angle - temporary works designer may be required to provide excavation support design - daily detailed inspection of excavation faces - potential for greater water inflow into excavation requiring removal of water using pumping - increased exclusion zone around excavation to avoid accidental loading of crest of slope Maintain hydrology of area as far as possible;
iii iv	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations; Use of experienced geotechnical staff for site investigation:
V	Use of experienced contractors and trained operators to carry out the work;
VI	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Temp.	Temp. Const. Comp. 2						
Grid Reference (Eastings, Northings):	607630	715110						
Distance to Watercourse (m)	> ′	> 150						
Min & Max Measured Peat Depth (m):	0.3 t	0.3 to 1.3						
Control Required:	No							

		Pre	e-Control Mea	sure Implem	entation			Pos	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 14.95 (u), 17.63 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Temporary Construction Compound 2
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Temp. (Temp. Const. Comp. 3						
Grid Reference (Eastings, Northings):	609113	715577						
Distance to Watercourse (m)	> '	150						
Min & Max Measured Peat Depth (m):	0.4 t	0.4 to 2.1						
Control Required:	Y	Yes						

		Pre-	Control Measu	ure Implem	entation			Pos	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 11.09 (u), 10.92 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Relatively deep peat	2	1	2	Negligible	Yes		1	1	1	Negligible

	Control Measures to be Implemented Prior to/and During Construction for Temporary Construction Compound 3
i ii iv v vi	Due to relatively deep peat at this temporary construction compound, additional construction measures such as the following may be required: - excavation side walls to be supported (eg. boulders, retaining wall units) or excavation face battered to shallow angle - temporary works designer may be required to provide excavation support design - daily detailed inspection of excavation faces - potential for greater water inflow into excavation requiring removal of water using pumping - increased exclusion zone around excavation to avoid accidental loading of crest of slope Maintain hydrology of area as far as possible; Installation of appropriate drainage measures to alleviate ingress of surface water into excavations; Use of experienced geotechnical staff for site investigation; Use of experienced contractors and trained operators to carry out the work; Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Temp. Const. Comp.					
Grid Reference (Eastings, Northings):	610305	715713				
Distance to Watercourse (m)	> '	150				
Min & Max Measured Peat Depth (m):	0.4 t	0.4 to 1.5				
Control Required:	No					

		Pre	Pre-Control Measure Implementation					Post	t-Control N	leasure Im	plementation			
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating			
1	FOS = 4.59 (u), 5.10 (d)	1	1	1	Negligible	No					1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible			
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible			
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable			
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible			
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible			
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible			
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable			
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable			
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable			
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable			

	Control Measures to be Implemented Prior to/and During Construction for Temporary Construction Compound 4
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Temp. (Temp. Const. Comp. 5					
Grid Reference (Eastings, Northings):	610858	718421					
Distance to Watercourse (m)	> ′	> 150					
Min & Max Measured Peat Depth (m):	0.8 t	0.8 to 1.1					
Control Required:	N N	No					

		Pre-	Pre-Control Measure Implementation					Pos	t-Control N	leasure Im	plementation		
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating		
1	FOS = 16.37 (u), 20.84 (d)	1	1	1	Negligible	No				1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible		
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible		
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible		
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible		
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible		
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable		
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable		

	Control Measures to be Implemented Prior to/and During Construction for Temporary Construction Compound 5
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Location:	Me	Met. Mast 1			
Grid Reference (Eastings, Northings):	60663	38 712525			
Distance to Watercourse (m)		> 150			
Min & Max Measured Peat Depth (m):	2	.7 to 3.5			
Control Required:		Yes			

		Pre-	Pre-Control Measure Implementation					Pos	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 7.64 (u), 6.55 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Relatively deep peat	3	1	3	Negligible	Yes		1	1	1	Negligible

	Control Measures to be Implemented Prior to/and During Construction for Met Mast 1
i	Due to relatively deep peat at this met mast location, additional construction measures such as the following may be required:
	- excavation side walls to be supported (eg. boulders, retaining wall units) or excavation face battered to shallow angle
	- temporary works designer may be required to provide excavation support design
	- daily detailed inspection of excavation faces
	- potential for greater water inflow into excavation requiring removal of water using pumping
	- increased exclusion zone around excavation to avoid accidental loading of crest of slope
ii	Maintain hydrology of area as far as possible;
111	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iv	Use of experienced geotechnical staff for site investigation;
v	Use of experienced contractors and trained operators to carry out the work;
vi	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties;
vil	Based on available ground investigation information, a piled met mast foundation may be required.

Location:	Met. Mast 2			
Grid Reference (Eastings, Northings):	609210	714421		
Distance to Watercourse (m)	> 150			
Min & Max Measured Peat Depth (m):	0.3 to 1.1			
Control Required:	No			

		Pre-	Pre-Control Measure Implementation					Pos	t-Control N	leasure Im	plementation
Ref.	Contributory/Qualitative Factors to Potential Peat Failure	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating	Control Required	Control measures to be implemented during construction	Prob (Note 2)	Impact (Note 3)	Risk	Risk Rating
1	FOS = 16.37 (u), 20.84 (d)	1	1	1	Negligible	No		1	1	1	Negligible
2	Evidence of sub peat water flow	1	1	1	Negligible	No		1	1	1	Negligible
3	Evidence of surface water flow	2	1	2	Negligible	No		1	1	1	Negligible
4	Evidence of previous failures/slips	0	1	0	Not Applicable	No		0	1	0	Not Applicable
5	Type of vegetation	2	1	2	Negligible	No		2	1	2	Negligible
6	General slope characteristics upslope/downslope from infrastructure location	1	1	1	Negligible	No	See Below	1	1	1	Negligible
7	Evidence of very soft/soft clay at base of peat	1	1	1	Negligible	No		1	1	1	Negligible
8	Evidence of mechanically cut peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
9	Evidence of quaking or buoyant peat	0	1	0	Not Applicable	No		0	1	0	Not Applicable
10	Evidence of bog pools	0	1	0	Not Applicable	No		0	1	0	Not Applicable
11	Other	0	1	0	Not Applicable	No		0	1	0	Not Applicable

	Control Measures to be Implemented Prior to/and During Construction for Met Mast 2
i	Maintain hydrology of area as far as possible;
ii	Installation of appropriate drainage measures to alleviate ingress of surface water into excavations;
iii	Use of experienced geotechnical staff for site investigation;
iv	Use of experienced contractors and trained operators to carry out the work;
v	Detailed ground investigation to determine peat, mineral soil and bedrock condition and properties.

Appendix C

Calculated FOS for Peat Slopes On Site









Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Undrained Analysis										
Turbine	Easting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Factor of Safety	for Load Condition	
No./Waypoint				strength	of Peat		Placed Fill Depth (m)			
			β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)	
T04	607027	745275	4	C.	10	0.5	4.5	co 33	22.02	
T01	607027	715375	1.5	6	10	1.4	2.4	16.38	9.55	
T03	606666	713348	1	6	10	1.4	2.4	24.56	14.33	
T04 T05	605958	713100	2	6	10	1.8	2.8	9.56	6.14	
T06	606452	712824	1.9	6	10	1.6	2.6	11.32	6.96	
T07	605724	714916	1	6	10	2.3	3.3	14.95	10.42	
T08 T09	605919 605954	715618	1	6	10	1.8	2.8	19.10	12.28	
T10	605094	714562	2	6	10	1.5	2.5	11.47	6.88	
T11	605273	714023	3.1	6	10	1.8	2.8	6.17	3.97	
T12	609600	714503	2	6	10	0.8	1.8	21.50	9.56	
T14	609982	714058	2	6	10	1.5	2.5	11.47	6.88	
T15 T16	610807	714384	2	6	10	0.7	1.7	24.58	10.12	
T17	610068	715972	3	6	10	0.8	1.8	14.35	6.38	
T18	609958	716616	4.1	6	10	0.8	1.8	10.52	4.67	
T20	609360	715724	1	6	10	1.2	2.2	28.65	15.63	
T21	609761	717087	2.9	6	10	1.6	2.6	7.42	4.57	
Substation	610966	718547	1	6	10	1.1	2.1	31.26	16.37	
TCC2	607630	715110	1	6	10	3.5	4.5	26.45	14.95	
TCC3	609113	715577	1	6	10	2.1	3.1	16.37	11.09	
TCC4 TCC5	610305 610858	715713	3	6	10	1.5	2.5	7.65	4.59	
MM1	606638	712525	1	6	10	3.5	4.5	9.82	7.64	
MM2	609210	714421	1	6	10	1.1	2.1	31.26	16.37	
FUS_1 FOS 2	606847	713320	3.2	6	10	0.9	1.9	11./1 17.64	5.61	
FOS_3	606800	713376	1.0	6	10	0.9	1.9	37.10	17.85	
FOS_4	606754	713358	1.7	6	10	0.9	1.9	22.37	10.75	
FOS_5	606660	713338	1.0	6	10	2.3	3.9	14.80	8.77	
FOS_7	606612	713310	1.0	6	10	2.3	3.3	14.80	10.34	
FOS_8	606564 606516	713296	1.0	6	10	0.9	1.9	36.83	17.78	
FOS_10	606468	713268	2.7	6	10	1.3	2.3	9.85	5.52	
FOS_11	606420	713254	1.0	6	10	1.6	2.6	21.99	13.41	
FOS_12 FOS_13	606372	713240	2.5	6	10	1.4	2.4	8.21	8.46 5.17	
FOS_14	606276	713212	2.9	6	10	2.1	3.1	5.72	3.88	
FOS_15	606228	713199	5.9	6	10	1.3	2.3	4.48	2.53	
FOS_10	606131	713172	3.4	6	10	1.4	2.4	6.16	3.82	
FOS_18	606083	713158	3.5	6	10	0.7	1.7	15.08	5.97	
FOS_19 FOS_20	606035 605987	713145 713131	1.0	6	10	1.1	2.1	30.66	16.21	
FOS_21	605939	713118	3.1	6	10	2.1	3.1	5.41	3.64	
FOS_22	605904	713108	5.1	6	10	4.2	5.2	1.62	1.31	
FOS 24	607141	714895	2.3	6	10	1.3	2.8	11.45	6.50	
FOS_25	607225	714860	1.0	6	10	2.6	3.6	13.20	9.54	
FOS_26	607230	714811 714761	4.1	6	10	1.5	2.5	5.68	3.40	
FOS_28	607227	714711	2.4	6	10	0.9	1.9	15.58	7.46	
FOS_29	607226	714661	3.3	6	10	0.4	1.4	27.24	7.52	
FOS_30	607224	714561	2.6	6	10	0.4	1.4	23.85	8.44	
FOS_32	607222	714511	6.4	6	10	0.7	1.7	7.40	3.11	
FOS_33 FOS_34	607221 607220	714461 714411	1.9	6	10	0.9	1.9	20.77	9.70 15.63	
FOS_35	607218	714361	1.0	6	10	1.5	2.5	23.01	13.78	
FOS_36	607217	714311	1.8	6	10	0.8	1.8	24.67	10.66	
FOS 38	607215	714261 714211	3.5	6	10	0.8	1.8 1.9	11.80	5.37	
FOS_39	607213	714161	4.9	6	10	1.5	2.5	4.57	2.77	
FOS_40	607212	714111	3.9	6	10	1.6	2.6	5.53	3.39	
FOS_42	607210	714001	1.0	6	10	1.8	2.5	19.55	12.46	
FOS_43	607208	713961	1.0	6	10	1.5	2.5	22.35	13.55	
FOS_44 FOS_45	607207 607206	/13911 713861	1.8	6	10 10	1.5	2.5 2.4	13.26 24,52	7.87 14.31	
FOS_46	607204	713811	1.0	6	10	2.1	3.1	16.65	11.22	
FOS_47	607182	713768	1.0	6	10	2.1	3.1	16.77	11.27	
FOS_48	607149	713694	1.0	6	10	2.0	3.0	10.25	6.78	
FOS_50	607081	713657	1.6	6	10	1.9	2.9	11.26	7.38	
FOS_51 FOS_52	607048 607014	713620	1.0	6	10	1.6	2.6	20.88	12.99 15.41	
FOS_53	606980	713546	0.8	6	10	1.0	2.0	44.81	21.91	
FOS_54	606947	713509	1.4	6	10	1.0	2.0	22.92	11.73	
FOS_55	606879	713472	2.1	6 6	10	0.6	1.6 2.0	<u>58.98</u> 16.03	8.07	
FOS_57	606862	713389	4.1	6	10	0.7	1.7	12.11	4.99	
FOS_58	606868	713340	1.0	6	10	0.8	1.8	43.07	19.12	
FOS_60	606882	713290	1.0	6	10	1.0	2.0	30.60	16.19	
EOS 61	000000	712101	1.0	6	10	2.0	2.0	17 (2)	11.05	

C	alculate	ed FoS o	f Natural	Peat Slope	es for Deri	inlough V	Vind Farm - U	ndrained An	alysis
Turbine	Easting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Factor of Safety	for Load Condition
No./Waypoint				strength	of Peat		Placed Fill Depth (m)		
				(1.5.)					
FOF (2)	000000	710140	β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)
FOS_62	606896	713142	2.2	6	10	2.6	3.6	5.88	4.25
FOS_64	606909	713042	1.0	6	10	2.9	3.9	11.81	8.79
FOS_65	606916	712993	1.0	6	10	3.3	4.3	10.27	7.91
FOS_66	606923	712943	1.0	6	10	2.3	3.3	14.88	10.39
FOS 68	606936	712894	1.0	6	10	2.2	3.2	15.70	10.78
FOS_69	606943	712795	1.9	6	10	2.6	3.6	7.01	5.06
FOS_70	606935	712747	1.4	6	10	2.9	3.9	8.28	6.16
FOS_71	606857	712/14	3.1	6	10	3.3	4.3	3.32	2.54
FOS 73	606816	712656	1.0	6	10	3.6	4.6	9.48	7.43
FOS_74	606775	712627	1.0	6	10	2.8	3.8	12.26	9.04
FOS_75	606735	712598	1.0	6	10	2.6	3.6	13.38	9.63
FOS_76	606653	712569	1.0	6	10	2.5	3.5	13.67	9.78
FOS_78	606641	712532	1.0	6	10	3.4	4.4	10.23	7.88
FOS_79	607185	714986	1.8	6	10	1.0	2.0	18.70	9.52
FOS_80	607187	714937	1.0	6	10	1.0	2.0	34.02	17.10
FOS 82	607109	714900	1.0	6	10	2.2	3.2	15.40	10.63
FOS_83	607062	714863	1.0	6	10	2.9	3.9	11.71	8.73
FOS_84	607016	714844	1.0	6	10	2.7	3.7	12.52	9.18
FOS 85	606969	714825	1.8	6	10	1.6	2.6	12.07	/.44
FOS_87	606877	714788	1.0	6	10	1.0	2.0	34.38	17.19
FOS_88	606830	714769	2.3	6	10	1.0	2.0	14.32	7.24
FOS_89	606784	714750	3.5	6	10	1.1	2.1	9.06	4.69
FOS 91	606691	714731	1.0	6	10	1.3	2.3	25.98	14.80
FOS_92	606644	714729	1.0	6	10	1.1	2.1	31.08	16.32
FOS_93	606622	714759	3.0	6	10	1.4	2.4	8.29	4.83
FOS_94	605897	715669	1.0	6	10	1.1	2.1	31.26	16.37
FOS 96	605897	715569	1.0	6	10	0.9	1.0	39.40	18.36
FOS_97	605897	715519	1.0	6	10	0.4	1.4	78.88	23.95
FOS_98	605897	715469	1.0	6	10	0.4	1.4	86.28	24.59
FUS_99	605897	715269	1.0	6	10	0.8	1.8	41.32	18.77
FOS 101	605897	715319	1.0	6	10	1.2	2.2	18.23	11.92
FOS_102	605897	715269	1.4	6	10	0.6	1.6	40.50	15.46
FOS_103	605897	715219	3.3	6	10	0.1	1.1	139.51	9.66
FOS_104 FOS_105	605897	715169	7.2	6	10	0.4	1.4	28.63	3.51
FOS_106	605897	715069	1.0	6	10	2.1	3.1	16.73	11.26
FOS_107	605897	715019	1.0	6	10	1.7	2.7	20.00	12.64
FOS_108	605919	714976	1.0	6	10	0.8	1.8	41.68	18.84
FOS_109 FOS_110	606014	714958	0.9	6	10	0.2	1.2	145.20	30.76
FOS_111	606064	714945	1.1	6	10	0.4	1.4	85.72	22.23
FOS_112	606113	714939	1.0	6	10	1.2	2.2	27.81	15.37
FOS_113	606212	714932	1.0	6	10	1.2	2.2	27.64	15.32
FOS 115	606262	714919	1.0	6	10	0.6	1.6	53.09	20.87
FOS_116	606312	714912	1.0	6	10	1.0	2.0	36.12	17.62
FOS_117	606361	714906	1.0	6	10	1.8	2.8	18.59	12.07
FOS_118 FOS_119	606460	714893	2.3	6	10	2.0	2.5	11.27	5.09
FOS_120	606510	714886	1.0	6	10	1.5	2.5	23.51	13.96
FOS_121	606559	714877	1.0	6	10	0.6	1.6	53.32	20.90
FOS_122 FOS_122	606594	714843	3.0 1 Q	6	10	1.0	2.0	11.49	5.71
FOS_124	606625	714748	2.1	6	10	2.1	3.1	7.67	5.21
FOS_125	606613	714701	1.0	6	10	1.5	2.5	22.87	13.74
FOS_126	606572	714674	1.0	6	10	1.7	2.7	20.23	12.74
FOS 127	606478	714658	1.0	6	10	1.5	2.5	22.41	13.57
FOS_129	606433	714618	1.9	6	10	1.8	2.8	10.13	6.51
FOS_130	606388	714596	1.0	6	10	2.0	3.0	17.39	11.55
FOS_131	606344	714574	1.0	6	10	2.0	3.0	16.86	11.31
FOS_132	606254	714530	1.0	6	10	1.5	2.5	4.95	13.70
FOS_134	606209	714508	3.6	6	10	1.0	2.0	10.04	4.90
FOS_135	606164	714486	2.4	6	10	1.8	2.8	7.79	5.04
FOS_136	606076	/14464 714440	4.9	6	10	1.9	2.9	3.66	2.41
FOS_138	606033	714413	2.6	6	10	0.9	1.9	14.09	6.78
FOS_139	605991	714386	1.0	6	10	0.4	1.4	82.38	24.26
FOS_140	605949	714360	3.3	6	10	1.1	2.1	9.86	5.10
FOS_141 FOS_142	605859	714333	1.0	6	10	1.5	2.5	25.07	14.02
FOS_143	605815	714344	2.6	6	10	0.5	1.5	26.01	8.70
FOS_144	605774	714372	1.7	6	10	1.3	2.3	14.98	8.57
FOS_145	605602	714401	1.0	6	10	1.5	2.5	23.10	13.82
FOS 147	605651	714459	1.0	6	10	1.2	2.2	29.66	13.94
FOS_148	605611	714488	1.9	6	10	1.6	2.6	11.53	7.06
FOS_149	605570	714516	1.0	6	10	3.0	4.0	11.34	8.53
FOS_150 FOS_151	605488	/14545 714574	1.0	6	10	2.7	3.7	12.92	9.39 14.24

С	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Undrained Analysis								
Turbine	Fasting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Eactor of Safety	for Load Condition
No./Waypoint	Lusting		otope	strength	of Peat	· cut Deptil	Placed Fill Depth (m)	. actor or barcely	
				_					
			β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)
FOS_152	605447	714603	1.0	6	10	1.4	2.4	24.60	14.34
FOS_153	605406	714632	1.0	6	10	1.1	2.1	32.02	16.58
FOS_154 FOS_155	605369	714664	2.4	6	10	0.9	1.9	16.04	7.56
FOS 156	605389	714752	3.8	6	10	0.5	1.7	20.14	6.28
FOS 157	605432	714777	1.1	6	10	0.8	1.8	36.19	16.41
FOS_158	605476	714801	1.7	6	10	1.0	2.0	20.78	10.20
FOS_159	605519	714826	1.0	6	10	1.1	2.1	32.21	16.63
FOS_160	605563	714850	1.8	6	10	0.6	1.6	32.21	11.86
FOS_161	605607	714875	1.0	6	10	0.6	1.6	53.90	20.99
FUS_162	605694	714899	1.0	6	10	1.1	2.1	31.25	10.37
FOS 164	605737	714948	1.0	6	10	1.5	2.5	23.50	13.96
FOS 165	605781	714972	7.0	6	10	0.2	1.2	30.81	4.30
FOS_166	605825	714997	1.0	6	10	1.0	2.0	32.82	16.79
FOS_167	605868	715022	1.0	6	10	1.9	2.9	18.30	11.94
FOS_168	605895	715062	1.0	6	10	2.0	3.0	17.58	11.63
FUS_169	605382	715088	1.0	6	10	1.4	2.4	24.68	14.37
FOS 171	605344	714681	1.0	6	10	0.9	1.5	36.68	17.75
FOS 172	605297	714695	1.8	6	10	1.0	2.0	19.28	9.51
FOS_173	605251	714676	4.6	6	10	0.9	1.9	8.17	3.90
FOS_174	605208	714651	4.3	6	10	1.1	2.1	7.08	3.76
FOS_175	605164	714627	3.0	6	10	1.4	2.4	8.19	4.79
FOS_176	605027	/14602	1.0	6	10	1.5	2.5	23.68	14.02
FOS_177	605033	714578	1.0	6	10	1.7	2.7	20.54	12.85
FOS 179	604990	714529	1.0	6	10	1.9	2.9	18.39	11.98
FOS_180	604962	714489	2.5	6	10	0.6	1.6	24.77	8.94
FOS_181	604971	714441	1.0	6	10	0.5	1.5	72.12	23.28
FOS_182	604998	714399	1.3	6	10	0.5	1.5	57.34	17.94
FOS_183	605026	714357	1.0	6	10	0.4	1.4	94.23	25.19
FOS_184	605053	714315	2.0	6	10	0.1	1.1	173.37	15.62
FOS_185	605108	714274	1.5	6	10	0.2	1.2	51.23	20.57
FOS 187	605136	714190	2.7	6	10	0.9	1.9	14.47	6.79
FOS 188	605164	714149	1.0	6	10	0.8	1.8	43.27	19.16
FOS_189	605191	714107	1.0	6	10	0.8	1.8	42.82	19.07
FOS_190	605219	714065	1.0	6	10	1.3	2.3	26.63	15.01
FOS_191	605246	714024	3.9	6	10	1.6	2.6	5.53	3.40
FOS_192	605274	713982	1.0	6	10	0.7	1./	49.16	20.23
FOS_193	608015	715068	1.0	6	10	1.3	2.3	25.90	14.77
FOS 195	607965	715063	11.3	6	10	1.4	2.4	2.30	1.32
FOS_196	607915	715059	1.0	6	10	1.6	2.6	21.91	13.38
FOS_197	607865	715054	1.9	6	10	2.0	3.0	9.20	6.11
FOS_198	607816	715050	2.2	6	10	1.2	2.2	12.86	7.09
FOS_199	607772	715070	1.0	6	10	1.6	2.6	21.49	13.22
FOS_200	607686	715104	8.4 1.0	6	10	1.2	3.1	27 55	1.34
FOS 202	607639	715080	1.0	6	10	0.4	1.4	81.40	24.17
FOS_203	607592	715063	0.7	6	10	0.6	1.6	90.10	32.16
FOS_204	607545	715046	0.7	6	10	0.9	1.9	58.49	26.96
FOS_205	607498	715029	1.0	6	10	0.9	1.9	41.23	19.02
FOS_206	607451	715012	1.0	6	10	0.8	1.8	42.65	19.04
FOS 202	607357	714995	3./ 1.0	6	10	0.7	1./	13.10	5.49
FOS 209	607310	714961	1.0	6	10	1.8	2.8	19.22	12.33
FOS_210	607263	714944	1.0	6	10	3.2	4.2	10.81	8.22
FOS_211	607214	714950	1.0	6	10	1.8	2.8	19.07	12.26
FOS_212	607184	714989	1.8	6	10	1.0	2.0	18.36	9.37
FOS_213	607168	715036	1.0	6	10	1.1	2.1	30.69	16.22
FOS 214	60712F	715083	1.0	6	10	1.1	2.1	32.18	10.62
FOS 216	607119	715178	1.5	6	10	1.0	2.0	17.55	8,98
FOS 217	607102	715225	1.0	6	10	1.5	2.5	23.47	13.95
FOS_218	607086	715272	1.0	6	10	2.4	3.4	14.18	10.04
FOS_219	607069	715319	1.0	6	10	1.7	2.7	21.36	13.31
FOS_220	607053	715367	1.2	6	10	0.9	1.9	32.33	15.17
FOS_221	607037	715414	1.0	6	10	0.6	1.6	58.77	21.69
FOS 222	607725	715430	1.0	6	10	1.9	1./ 2 Q	49.25	20.25
FOS 224	607774	715113	3.0	6	10	2.2	3.2	5.17	3.55
FOS_225	607823	715121	2.4	6	10	1.3	2.3	10.86	6.17
FOS_226	607873	715129	1.0	6	10	2.7	3.7	12.95	9.41
FOS_227	607922	715136	1.0	6	10	1.9	2.9	17.99	11.81
FOS_228	607932	715138	1.0	6	10	1.8	2.8	19.57	12.47
FOS_229	607004	714964	1.0	6	10	0.5	1.5	64.31 174.36	22.40
FOS 231	608045	715003	1.0	6	10	0.2	1.2	246.13	30.17
FOS 232	608087	715022	1.0	6	10	0.2	1.2	175.07	28.74
FOS_233	608049	715053	1.0	6	10	0.9	1.9	36.68	17.75
FOS_234	608003	715067	9.1	6	10	1.3	2.3	2.86	1.63
FOS_235	607965	715038	1.0	6	10	0.9	1.9	38.67	18.20
FOS_236	607962	714989	17.4	6	10	0.1	1.1	15.11	1.84
FOS 237	609729	717166	1.0	6	10	0.5	1.5	64.62 16.04	22.44
FOS 239	609728	717116	1.4	6	10	0.8	1.8	30.92	13.52
FOS 240	609744	717069	2.3	6	10	1.4	2.4	11.03	6.36
FOS 241	609761	717022	3.1	6	10	1.0	2.0	11.39	5.63

C	alculate	ed FoS o	f Natura	Peat Slope	es for Deri	inlough V	Vind Farm - U	ndrained An	alysis
Turbine	Easting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Factor of Safety	for Load Condition
No./Waypoint	Ū			strength	of Peat		Placed Fill Depth (m)		
				(1.5.)					
505 343	600777	74 60 75	β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)
FOS_242 FOS_243	609794	716975	2.2	6	10	1.0	2.1	14.18	8.34
FOS_244	609811	716881	3.2	6	10	1.1	2.1	10.10	5.21
FOS_245	609833	716836	2.9	6	10	1.1	2.1	11.10	5.72
FOS_246	609854	716791	5.0	6	10	1.7	2.7	3.99	2.52
FOS 248	609808	716711	0.5	6	10	0.9	1.9	70.28	34.22
FOS_249	609764	716687	0.5	6	10	1.2	2.2	62.75	34.17
FOS_250	609720	716663	0.4	6	10	0.8	1.8	102.94	46.77
FOS_251	609677	716638	1.0	6	10	0.8	1.8	40.61	18.62
FOS 253	609590	716589	0.6	6	10	1.5	2.5	40.19	24.07
FOS_254	609546	716565	0.7	6	10	1.4	2.4	31.87	18.85
FOS_255	609502	716540	1.8	6	10	1.3	2.3	14.59	8.32
FOS_256	609459	716516	1.0	6	10	1.5	2.6	20.93	13.01
FOS_258	609371	716467	0.8	6	10	1.1	2.1	39.29	20.50
FOS_259	609328	716443	0.7	6	10	1.0	2.0	47.79	24.44
FOS_260	609284	716418	1.1	6	10	1.0	2.0	32.74	16.08
FOS 262	609245	716336	1.4	6	10	0.5	1.5	61.08	19.47
FOS_263	609253	716286	0.9	6	10	0.4	1.4	108.57	29.24
FOS_264	609260	716237	1.0	6	10	0.6	1.6	61.65	22.07
FOS 265	609268	716187	1.1	6	10	U.6 1 0	1.6	46.21	18.20
FOS_267	609283	716088	0.9	6	10	0.7	1.7	55.15	23.19
FOS_268	609290	716039	3.2	6	10	1.8	2.8	5.89	3.81
FOS_269	609298	715990	1.0	6	10	2.6	3.6	13.12	9.49
FOS_270 FOS_271	609313	715940	2.5	6	10	1.5 0.7	2.5	8.99 21.77	5.42
FOS_272	609320	715841	1.0	6	10	0.5	1.5	75.21	23.60
FOS_273	609328	715792	1.0	6	10	0.8	1.8	40.55	18.61
FOS_274	609335	715742	1.0	6	10	1.3	2.3	26.48	14.96
FOS_275	609343	715645	1.0	6	10	0.9	1.5	38.44	18.15
FOS_277	609292	715617	3.1	6	10	1.4	2.4	7.88	4.61
FOS_278	609246	715598	1.0	6	10	1.0	2.0	35.11	17.37
FUS_279	609200	715578	3.1	6	10	1.8	2.8	6.20	3.96
FOS 281	609108	715539	1.0	6	10	1.5	2.6	21.48	13.22
FOS_282	609062	715520	4.1	6	10	1.0	2.0	8.38	4.22
FOS_283	609016	715501	1.9	6	10	1.2	2.2	15.51	8.37
FOS_284 FOS_285	608967	715490	2.4	6	10	0.9	2.1	13.16	6.86
FOS_286	608868	715474	1.4	6	10	0.5	1.5	49.15	16.13
FOS_287	608819	715466	1.0	6	10	0.1	1.1	329.35	31.13
FOS_288	608774	715445	1.0	6	10	0.1	1.1	513.97	32.23
FOS 290	608695	715384	1.0	6	10	0.0	1.0	8596.11	34.25
FOS_291	608655	715354	1.0	6	10	0.0	1.0	85961.13	34.37
FOS_292	608614	715326	1.0	6	10	0.0	1.0	57307.42	34.36
FOS_293	608573	715297	1.0	6	10	0.0	1.0	42980.56	34.36
FOS_295	608502	715226	1.0	6	10	0.0	1.0	343844.50	34.38
FOS_296	608467	715191	1.0	6	10	0.0	1.0	10745.14	34.27
FOS_297	608432	715155	5.4	6	10	0.0	1.0	9103.98	6.37
FOS 299	608354	715096	1.0	6	10	0.0	1.0	34384.45	34.35
FOS_300	608307	715079	3.1	6	10	0.0	1.0	15919.30	11.14
FOS_301	608260	715061	23.2	6	10	0.0	1.0	295.71	1.65
FOS 302	608165	/15044 715029	5.0	6	10	0.1	1.1 1 1	124.98	6.58 30.24
FOS_304	608117	715019	1.0	6	10	0.1	1.1	667.66	32.70
FOS_305	608077	715010	2.3	6	10	0.0	1.0	410.49	14.49
FOS_306	609716	715253	2.9	6	10	1.1	2.1	11.13	5.73
FOS 308	609628	715300	0.5	6	10	0.4	1.4	94.52	41.82
FOS_309	609583	715323	0.5	6	10	0.4	1.4	182.36	53.15
FOS_310	609539	715347	1.0	6	10	0.5	1.5	70.33	23.09
FOS_311 FOS_312	609495 609451	715370	2.3	6	10	0.4	1.4	33.16	10.17
FOS_313	609411	715423	1.0	6	10	0.5	1.5	73.33	23.41
FOS_314	609395	715470	1.0	6	10	0.3	1.3	117.07	26.58
FOS_315	609384	715518	1.0	6	10	0.4	1.4	77.22	23.79
FOS_316 FOS_317	609345	715567	3.1 6.7	6	10	1.0	2.0	4,34	2.37
FOS_318	609297	715617	3.1	6	10	1.8	2.8	6.25	4.00
FOS_319	609283	715613	4.1	6	10	1.6	2.6	5.31	3.25
FOS_320	609680	714336	3.9	6	10	0.6	1.6	15.61	5.60
FOS 322	609641	714426	1.0	6	10	0.4	1.4	84.46	24.44
FOS_323	609630	714475	1.0	6	10	0.5	1.5	62.79	22.22
FOS_324	609618	714523	1.0	6	10	0.7	1.7	47.02	19.86
FOS_325	609607 609595	714572	1.0	6	10	0.7	1.7	49.07	20.22
FOS 327	609584	714669	1.0	6	10	2.2	3.2	15.56	10.71
FOS_328	609572	714718	7.5	6	10	2.7	3.7	1.74	1.26
FOS_329	609561	714767	11.0	6	10	1.7	2.7	1.94	1.21
FOS 331	609538	/14815 714864	2.3	6 6	10	0.8	1.8	2.83	1.30

С	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Undrained Analysis								
Turbine	Fasting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Factor of Safety	for Load Condition
No./Waypoint	Lusting		otope	strength	of Peat	· cut Deptil	Placed Fill Depth (m)	ruttor or ourcey	
				_					
			β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)
FOS_332	609527	714913	1.0	6	10	0.9	1.9	39.61	18.41
FOS_333	609515	714961	1.0	6	10	0.9	1.9	36.29	17.65
FOS_335	609492	715010	1.0	6	10	0.6	1.7	40.08	19.80
FOS_336	609481	715107	1.0	6	10	0.9	1.9	37.77	18.00
FOS_337	609469	715156	1.2	6	10	0.9	1.9	33.61	15.45
FOS_338	609458	715205	1.0	6	10	1.0	2.0	35.57	17.48
FOS_339	609446	715253	2.8	6	10	0.8	1.8	15.69	6.89
FOS_340	609435	715302	1.0	6	10	0.7	1./	48.17	20.06
FOS 342	609412	715399	3.1	6	10	0.9	1.5	11.90	5.70
FOS 343	609401	715445	1.0	6	10	0.4	1.4	79.59	24.01
FOS_344	610491	714779	1.0	6	10	1.0	2.0	36.03	17.59
FOS_345	610540	714780	1.0	6	10	1.6	2.6	22.01	13.42
FOS_346	610576	714748	1.0	6	10	0.9	1.9	38.42	18.15
FOS_347	610581	714699	1.0	6	10	1.1	2.1	32.44	15.09
FOS 349	610569	714600	1.0	6	10	1.0	2.0	33.11	16.87
FOS_350	610563	714550	1.0	6	10	1.2	2.2	28.48	15.58
FOS_351	610557	714500	1.0	6	10	1.4	2.4	24.62	14.35
FOS_352	610551	714451	1.0	6	10	1.5	2.5	22.30	13.53
FOS_353	610545	/14401	1.0	6	10	1.4	2.4	23.72	14.04
FOS 355	610533	714351	1.0	6	10	1.4	2.4	23.//	14.05
FOS_356	610517	714255	1.8	6	10	1.4	2.4	13.34	7.80
FOS_357	610475	714229	1.0	6	10	1.1	2.1	30.60	16.19
FOS_358	610429	714211	1.0	6	10	1.3	2.3	27.09	15.15
FOS_359	610382	714193	2.5	6	10	0.7	1.7	20.96	8.39
FOS 360	610336	714174	1.0	6	10	0.1	1.1	512.02	30.97
FOS 362	610243	714138	1.0	6	10	0.5	1.3	41.73	18.85
FOS_363	610196	714119	2.2	6	10	0.9	1.9	18.35	8.49
FOS_364	610150	714101	1.0	6	10	0.9	1.9	40.12	18.52
FOS_365	610103	714083	1.0	6	10	0.8	1.8	43.57	19.22
FOS_366	610056	714064	1.0	6	10	1.0	2.0	33.82	17.05
FOS_367	610010	714046	1.0	6	10	0.9	1.9	37.00	17.82
FOS_369	609915	714029	1.9	6	10	1.7	2.0	35.90	17.56
FOS 370	609883	714077	1.0	6	10	0.6	1.6	53.15	20.88
FOS_371	609852	714117	7.9	6	10	0.5	1.5	9.71	3.04
FOS_372	609821	714156	1.4	6	10	0.5	1.5	45.19	15.68
FOS_373	609790	714195	1.0	6	10	2.7	3.7	12.72	9.28
FUS_374	609759	714235	6.3	6	10	0.4	1.4	13.46	3.92
FOS 376	609697	714313	3.6	6	10	0.2	1.2	34.41	7.48
FOS_377	609664	714350	1.0	6	10	1.1	2.1	30.26	16.09
FOS_378	609616	714363	1.6	6	10	0.5	1.5	39.71	13.92
FOS_379	609567	714370	1.0	6	10	0.5	1.5	69.97	23.06
FOS_380	609517	714378	1.0	6	10	0.5	1.5	69.25	22.98
FOS_381 FOS_382	609468	714385	1.0	6	10	0.2	1.2	52.02	29.78
FOS_383	609369	714400	1.0	6	10	0.8	1.8	41.77	18.86
FOS_384	609319	714407	2.2	6	10	1.9	2.9	8.24	5.42
FOS_385	609270	714414	1.0	6	10	2.4	3.4	14.62	10.26
FOS_386	609221	714422	1.0	6	10	1.2	2.2	27.55	15.29
FUS_387	610862	714422	1.0	6	10	0.9	1.9	38.63	18.19
FOS 389	610802	714364	8.4	6	10	0.9	1.9	4.53	2.17
FOS_390	610769	714346	1.0	6	10	0.5	1.5	75.54	23.63
FOS_391	610723	714327	1.0	6	10	0.9	1.9	38.44	18.15
FOS_392	610676	714309	1.0	6	10	0.8	1.8	42.97	19.10
FOS_393	610630	714290	0.7	6	10	1.3	2.3	36.85	21.06
FOS 394	610545	714290	1.0	6	10	1.4	2.4	32.32 21.93	13.39
FOS_396	610541	714372	1.0	6	10	1.1	2.1	31.29	16.38
FOS_397	610541	714373	1.0	6	10	1.1	2.1	31.59	16.46
FOS_398	610070	716252	1.0	6	10	0.4	1.4	91.04	24.96
FOS_399	610088	716206	1.5	6	10	0.8	1.8	28.83	12.55
FOS 400	610105	716159	1.6	6	10	0.5	1.5	40.51	14.02
FOS_401 FOS_402	610125	716065	3.3	6	10	0.7	1.7	40.23	6,94
FOS_403	610158	716018	1.0	6	10	0.1	1.1	373.34	31.48
FOS_404	610176	715972	4.3	6	10	0.9	1.9	9.03	4.25
FOS_405	610193	715925	2.9	6	10	1.6	2.6	7.23	4.48
FOS_406	610211	715878	1.0	6	10	0.8	1.8	42.21	18.95
FOS 407	610246	715794	1.9	6	10	0.7	1./	25.85	2 58 10.68
FOS 409	610263	715737	2.1	6	10	0.5	1.5	30.94	10.65
FOS_410	610281	715691	1.0	6	10	0.6	1.6	61.04	21.99
FOS_411	610299	715644	1.0	6	10	0.9	1.9	36.29	17.66
FOS_412	610316	715597	1.0	6	10	1.0	2.0	35.90	17.56
FOS_413	610334	715550	1.8	6	10	0.6	1.6	31.89	11.81
FOS 414	610351	715457	1.1	ь 6	10	0.1	1.1	283.58 76.87	28.42
FOS 416	610386	715410	2.5	6	10	0.5	1.5	27.09	9.22
FOS_417	610404	715363	1.0	6	10	0.9	1.9	36.89	17.80
FOS_418	610421	715316	3.3	6	10	0.7	1.7	15.28	6.24
FOS_419	610439	715269	4.1	6	10	1.0	2.0	8.23	4.15
FOS_420	610457 610474	/15222	4.9	6	10	0.8	1.8	9.08	3.99

C	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Undrained Analysis								
Turbine	Easting	Northing	Slope	Undrained shear	Bulk unit weight	Peat Depth	Surcharge Equivalent	Factor of Safety	for Load Condition
No./Waypoint				strength	of Peat		Placed Fill Depth (m)		
				(1.5.)					
505 422	640402	745420	β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)
FOS_422 FOS_423	610492	715129	4.3 5.5	6	10	0.6	1.7	10.82	3.96
FOS_424	610527	715035	3.3	6	10	0.9	1.9	11.66	5.49
FOS_425	610544	714988	1.3	6	10	1.3	2.3	19.36	11.12
FOS_426	610562	714942	1.0	6	10	2.6	3.6	13.06	9.47
FOS_427	610579	714895	1.0	6	10	1.6	2.6	21.04	13.05
FOS 429	610558	714846	1.0	6	10	1.5	2.5	22.43	13.58
FOS_430	610512	714787	1.0	6	10	1.1	2.1	30.16	16.07
FOS_431	610465	714768	1.0	6	10	1.2	2.2	29.64	15.92
FOS_432	610419	714750	1.0	6	10	1.1	2.1	30.82	16.25
FOS 434	610326	714732	0.6	6	10	0.9	1.9	66.80	31.61
FOS_435	610279	714695	0.9	6	10	0.7	1.7	55.45	23.24
FOS_436	610275	714693	0.9	6	10	0.4	1.4	91.51	27.84
FOS_437	609810	716712	2.2	6	10	1.0	2.0	14.86	7.56
FOS_438	609857	716725	4.9	6	10	0.9	1.9	38.55	13.47
FOS_440	609920	716656	1.0	6	10	0.1	1.1	311.45	30.97
FOS_441	609938	716609	1.0	6	10	0.3	1.3	106.75	26.01
FOS_442	609955	716562	4.3	6	10	1.3	2.3	6.34	3.53
FOS_443	609972	716516	2.8	6	10	0.7	1./	18.57	/.39
FOS_445	610007	716422	1.4	6	10	0.9	1.9	39.31	18.60
FOS_446	610024	716375	0.9	6	10	0.6	1.6	60.46	23.15
FOS_447	610042	716328	1.3	6	10	0.6	1.6	42.17	16.57
FOS_448	610060	/16281	1.0	6	10	0.7	1.7	49.49	20.29
FOS 450	610070	716184	1.0	6	10	1.1	2.1	31.92	16.55
FOS_451	610064	716134	3.4	6	10	0.7	1.7	14.28	5.93
FOS_452	610058	716085	1.0	6	10	0.5	1.5	74.90	23.57
FOS_453	610053	716035	1.0	6	10	0.4	1.4	81.52	24.18
FOS_454 FOS_455	610047	715985	1.0	6	10	0.7	1./	49.35	20.26
FOS 456	610041	715925	2.3	6	10	0.4	1.4	27.68	9.58
FOS_457	610903	718466	1.0	6	10	1.0	2.0	33.79	17.04
FOS_458	610855	718452	1.0	6	10	0.9	1.9	37.40	17.92
FOS_459	610807	718439	1.0	6	10	0.8	1.8	41.90	18.89
FOS_460	610759	718425	2.1	6	10	1.0	2.0	13.69	7.43
FOS_462	610668	718387	1.0	6	10	1.4	2.4	25.31	14.58
FOS_463	610653	718341	2.6	6	10	1.2	2.2	10.71	5.89
FOS_464	610668	718293	1.0	6	10	1.1	2.1	30.29	16.10
FOS_465	610586	718246	2.3 4.1	6	10	1.1	2.1	14.18	4.25
FOS_467	610721	718153	9.6	6	10	1.0	2.0	3.65	1.83
FOS_468	610739	718106	3.8	6	10	1.0	2.0	9.09	4.56
FOS_469	610702	718104	1.0	6	10	0.8	1.8	44.84	19.46
FOS_470	610657	718125	1.0	6	10	0.4	1.4	82.91	24.31
FOS_471 FOS_472	610564	718143	1.0	6	10	0.5	1.5	92.83	25.04
FOS_473	610522	718105	3.0	6	10	0.2	1.2	68.43	9.74
FOS_474	610480	718078	3.5	6	10	0.0	1.0	413.08	9.64
FOS_475	610462	718038	3.0	6	10	0.1	1.1	181.91	10.88
FOS_476	610482	717992	2.4	6	10	0.2	1.2	587 77	32.48
FOS_478	610509	717899	2.7	6	10	0.1	1.1	158.59	11.61
FOS_479	610488	717853	4.9	6	10	0.4	1.4	18.08	5.10
FOS_480	610467	717808	10.1	6	10	0.7	1.7	5.14	2.07
FOS_481	610445	/17763	14.1 12 ^c	6	10	1.0	2.0	2.64	1.30
FOS 483	610392	717678	7.5	6	10	1.0	2.0	4.65	2.32
FOS_484	610355	717646	10.1	6	10	0.8	1.8	4.46	1.95
FOS_485	610315	717616	2.5	6	10	0.5	1.5	26.30	8.99
FOS_486	610283	717577	2.5	6	10	0.4	1.4	34.09	9.91
FOS 488	610249	717484	2.3	6	10	0.4	1.4	32.77	10.30
FOS_489	610209	717456	1.0	6	10	0.5	1.5	64.71	22.45
FOS_490	610164	717436	1.0	6	10	0.6	1.6	56.01	21.31
FOS_491	610118	717415	1.8	6	10	0.7	1.7	28.32	11.50
FOS 492	610073	717374	1.0	6	10	0.8	1.8 1.8	45.59	19.60
FOS_494	609979	717368	1.6	6	10	1.0	2.0	22.06	10.87
FOS_495	609931	717362	2.5	6	10	1.3	2.3	10.66	5.99
FOS_496	609885	717343	4.2	6	10	1.6	2.6	5.32	3.24
FOS_497	609860	/17382	1.0	6	10	1.8	2.8	18.85	12.18
FOS 499	609800	717443	1.3	6	10	2.0	3.0	13.06	8.70
FOS_500	609750	717439	2.9	6	10	1.7	2.7	7.03	4.40
FOS_501	609700	717434	2.2	6	10	1.2	2.2	13.16	7.18
FOS_502	609660	717412	2.1	6	10	0.9	1.9	18.95	8.75
FOS 504	609687	717315	1.0	6	10	0.9	1.9	38.45	18.15
FOS_505	609700	717267	1.0	6	10	1.0	2.0	35.56	17.48
FOS_506	609713	717219	1.0	6	10	1.0	2.0	35.47	17.46
FOS_507	609727	717171	3.4	6	10	0.8	1.8	12.53	5.62
FOS 517	6109728	719039	1.0	6 6	10	0.8	1.8	43.10	19.13
FOS 518	610955	718992	1.0	6	10	0.2	1.2	206.64	29.48
EOS 519	610933	718947	10	6	10	0.3	13	103.66	25.82

Ca	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Undrained Analysis											
Turbine No./Waypoint	Easting	Northing	Slope	Undrained shear strength	Bulk unit weight of Peat	Peat Depth	Surcharge Equivalent Placed Fill Depth (m)	Factor of Safety	for Load Condition			
			β (deg)	c _u (kPa)	γ (kN/m³)	(m)	Condition (2)	Condition (1)	Condition (2)			
FOS 520	610908	718904	1.0	6	10	0.5	1.5	71.43	23.21			
FOS 521	610883	718861	1.0	6	10	0.6	1.6	56.57	21.39			
FOS 522	610858	718817	1.0	6	10	0.5	1.5	65.10	22.50			
FOS 523	610833	718774	1.0	6	10	0.3	1.3	115.97	26.52			
FOS 524	610808	718731	2.7	6	10	0.5	1.5	25.60	8.41			
FOS 525	610782	718688	2.6	6	10	0.9	1.9	15.07	7.00			
FOS 526	610757	718644	2.1	6	10	1.3	2.3	12.15	6.95			
FOS 527	610735	718600	1.0	6	10	1.8	2.8	19.14	12.29			
FOS 528	610719	718553	3.7	6	10	1.8	2.8	5.18	3.34			
FOS_529	610703	718505	3.6	6	10	1.5	2.5	6.35	3.82			
FOS 530	610714	718458	5.0	6	10	1.3	2.3	5.53	3.08			
FOS_531	610755	718431	1.0	6	10	1.0	2.0	34.27	17.16			
FOS_532	610793	718435	1.0	6	10	0.8	1.8	41.48	18.80			
FOS_533	610790	718434	1.0	6	10	0.8	1.8	40.96	18.69			
FOS_534	610827	718467	1.0	6	10	0.9	1.9	39.82	18.45			
FOS_535	610872	718488	1.0	6	10	1.0	2.0	36.05	17.60			
FOS_536	610895	718494	1.0	6	10	1.0	2.0	34.64	17.26			
FOS_537	605839	714975	5.5	6	10	1.1	2.1	5.51	2.94			
FOS_538	605889	714968	4.1	6	10	0.8	1.8	10.95	4.75			
FOS_539	605337	714723	2.1	6	10	0.8	1.8	19.50	8.86			
FOS_540	606613	714688	1.0	6	10	1.2	2.2	27.78	15.36			
FOS_541	606656	714701	1.0	6	10	1.0	2.0	33.73	17.03			
FOS_542	606241	713131	3.3	6	10	2.3	3.3	4.48	3.14			
FOS_543	606969	713109	1.0	6	10	2.8	3.8	12.35	9.09			
FOS_544	606835	713406	3.2	6	10	0.2	1.2	55.03	8.99			
FOS_545	607232	714892	1.0	6	10	2.9	3.9	11.97	8.88			
FOS_546	607241	714921	1.0	6	10	3.0	4.0	11.44	8.59			
FOS_547	610560	714263	1.0	6	10	1.3	2.3	25.79	14.74			
FOS_548	610593	714791	1.0	6	10	0.6	1.6	55.03	21.16			
FOS_549	609355	715638	1.0	6	10	1.1	2.1	32.02	16.58			
FOS_550	609875	716752	5.1	6	10	1.3	2.3	5.37	2.99			
FOS_551	610959	718907	2.0	6	10	0.1	1.1	133.99	15.21			
FOS_552	610913	718829	1.0	6	10	0.4	1.4	88.87	24.79			
FOS_553	610915	718765	1.0	6	10	0.2	1.2	148.27	27.91			
FOS_554	610917	718691	1.0	6	10	0.5	1.5	69.13	22.96			
FOS_555	610918	718618	1.0	6	10	1.0	2.0	35.52	17.47			
FOS_556	605828	715369	11.5	6	10	1.3	2.3	2.29	1.31			
FOS_557	607064	715123	1.0	6	10	1.0	2.0	33.69	17.02			
FOS_558	605021	714220	1.5	6	10	0.8	1.8	28.94	12.58			

Minimum =	1.62	1.21
Maximum =	343844.50	53.15
Average =	1194.54	13.76

Notes:

 Notes:

 (1) Assuming a bulk unit weight for peat of 10kN/m³

 (2) Assuming a surcharge equivalent to fill depth of 1m of peat i.e. 10kPa.

 (3) Stope inclination (β) based on site readings and topographical survey data.

 (4) A lower bound undrained shear strength, cu for the peat of 6kPa was selected for the assessment. It should be noted that a cu of 6kPa for the peat is considered a conservative value for the analysis and is not representative of all peat present across the site. In reality the peat has a significantly higher undrained strength.

 (5) Peat depths based on probes carried out by FT, HES and from the GPR survey carried out by BnM.

 (6) For load conditions see report text.

Calc	ulated	FoS of	Natural Pe	at Slopes	Drained Analysis				
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of Peat	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety	or Load Condition
	α (deg)	c' (kPa)	γ (kN/m³)	$\gamma_w (kN/m^3)$	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
								100% Water	100% Water
T01	1	4	10.0	10.0	0.5	25	1.5	45.85	33.09
T02	1.5	4	10.0	10.0	1.4	25	2.4	10.92	13.79
T03 T04	1	4	10.0	10.0	1.4	25	2.4	16.37 6.37	20.68
T05	1	4	10.0	10.0	2.3	25	3.3	9.97	15.04
T06	1.9	4	10.0	10.0	1.6	25	2.6	7.54	10.05
T08	1	4	10.0	10.0	1.8	25	2.8	12.73	15.04
T09	1	4	10.0	10.0	1.7	25	2.7	13.48	18.38
T10 T11	3.1	4	10.0	10.0	1.5	25	2.5	4.12	5.72
T12	1	4	10.0	10.0	0.8	25	1.8	28.65	27.58
T13 T14	2	4	10.0	10.0	0.8	25	1.8	14.34	13.79 9 93
T15	2	4	10.0	10.0	0.7	25	1.7	16.38	14.60
T16	1	4	10.0	10.0	0.6	25	1.6	38.20	31.02
T18	4.1	4	10.0	10.0	0.8	25	1.8	7.01	6.73
T19	1	4	10.0	10.0	1.2	25	2.2	19.10	22.56
T20	1 2.9	4	10.0	10.0	1.1	25	2.1 2.6	<u>20.84</u> 4.95	23.64 6.59
Substation	1	4	10.0	10.0	1.1	25	2.1	20.84	23.64
TCC1 TCC2	1	4	10.0 10.0	10.0	3.3 1.3	25 25	4.3 2.3	6.95 17.63	11.54 21.58
TCC3	1	4	10.0	10.0	2.1	25	3.1	10.92	16.01
TCC4	3	4	10.0	10.0	1.5	25	2.5	5.10	6.62 23.64
MM1	1	4	10.0	10.0	3.5	25	4.5	6.55	11.03
MM2	1	4	10.0	10.0	1.1	25	2.1	20.84	23.64
FOS_1 FOS_2	3.2	4	10.0 10.0	10.0	0.9	25 25	1.9 1.7	7.81 11.76	8.08 10.83
FOS_3	1.0	4	10.0	10.0	0.9	25	1.9	24.74	25.76
FOS_4	1.7	4	10.0	10.0	0.9	25	1.9	14.91	15.52
FOS_6	1.0	4	10.0	10.0	2.9	25	3.9	7.85	12.66
FOS_7	1.0	4	10.0	10.0	2.3	25	3.3	9.86	14.93
FOS_8	1.0	4	10.0	10.0	1.2	25	2.2	12.66	14.78
FOS_10	2.7	4	10.0	10.0	1.3	25	2.3	6.57	7.95
FOS_11 FOS_12	1.0	4	10.0	10.0	1.6	25	2.6	14.66 9.78	19.36 12.22
FOS_13	2.5	4	10.0	10.0	1.7	25	2.7	5.47	7.46
FOS_14	2.9	4	10.0	10.0	2.1	25	3.1	3.82	5.59
FOS_16	1.0	4	10.0	10.0	1.4	25	2.4	16.45	20.74
FOS_17	3.4	4	10.0	10.0	1.6	25	2.6	4.11	5.50
FOS_18 FOS_19	3.5	4	10.0	10.0	0.7	25	2.1	20.44	23.40
FOS_20	1.0	4	10.0	10.0	1.0	25	2.0	22.26	24.46
FOS_21 FOS_22	3.1	4	10.0	10.0	2.1	25	3.1	3.61	5.25
FOS_23	1.0	4	10.0	10.0	1.8	25	2.8	12.96	17.93
FOS_24	2.3	4	10.0	10.0	1.3	25	2.3	7.63	9.37
FOS_26	4.1	4	10.0	10.0	1.5	25	2.5	3.79	4.90
FOS_27	1.7	4	10.0	10.0	1.0	25	2.0	12.77	14.13
FOS_28 FOS_29	2.4	4	10.0	10.0	0.9	25	1.9	10.39	10.76
FOS_30	1.0	4	10.0	10.0	0.4	25	1.4	58.02	35.58
FOS_31 FOS_32	2.6	4 4	10.0	10.0	0.5	25 25	1.5	15.90 4.94	12.18 4.47
FOS_33	1.9	4	10.0	10.0	0.9	25	1.9	13.85	14.00
FOS_34	1.0	4	10.0	10.0	1.2	25	2.2	19.10	22.56
FOS_36	1.0	4	10.0	10.0	0.8	25	1.8	16.45	15.38
FOS_37	3.5	4	10.0	10.0	0.8	25	1.8	7.87	7.75
FOS 39	3.7	4	10.0	10.0	1.5	25	2.5	3.05	3.98
FOS_40	3.9	4	10.0	10.0	1.6	25	2.6	3.69	4.88
FOS_41 FOS_42	1.0 1.0	4	10.0 10.0	10.0	1.5 1.8	25 25	2.5 2.8	15.05 13.03	19.68 17.99
FOS_43	1.0	4	10.0	10.0	1.5	25	2.5	14.90	19.55
FOS_44	1.8	4 A	10.0	10.0	1.5	25	2.5	8.84	11.36
FOS_46	1.0	4	10.0	10.0	2.1	25	3.1	11.10	16.19
FOS_47	1.0	4	10.0	10.0	2.1	25	3.1	11.18	16.27
FOS_48 FOS_49	1.0	4	10.0	10.0	2.0	25	3.0	<u>11.45</u> 6.83	16.53 9.78
FOS_50	1.6	4	10.0	10.0	1.9	25	2.9	7.51	10.66
FOS_51 FOS_52	1.0	4	10.0	10.0	1.6	25 25	2.6	13.92	18.76 22.25
FOS_53	0.8	4	10.0	10.0	1.0	25	2.0	29.87	31.63
FOS_54	1.4	4	10.0	10.0	1.0	25	2.0	15.28	16.93
FOS_55	2.1	4	10.0	10.0	1.0	25	2.0	39.32 10.69	51.36 11.64
FOS_57	4.1	4	10.0	10.0	0.7	25	1.7	8.07	7.19
+OS_58 FOS_59	1.0	4	10.0	10.0	0.8	25	1.8	28.71	27.60 3.81
FOS_60	1.0	4	10.0	10.0	1.1	25	2.1	20.40	23.38
FOS_61	1.0	4	10.0	10.0	2.0	25	3.0	11.75	16.82
FOS_63	2.2	4	10.0	10.0	2.6	25	3.6	3.92	6.14
FOS_64	1.0	4 A	10.0	10.0	2.9	25	3.9	7.87	12.69 11 41

Calc	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farr				Wind Farm	n - Drained Analysis			
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of Peat	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety f	or Load Condition
	α (deg)	c' (kPa)	γ (kN/m ³)	γ _w (kN/m³)	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
FOS 66	1.0	4	10.0	10.0	2.3	25	3.3	100% Water 9.92	100% Water 14.99
FOS_67	2.3	4	10.0	10.0	1.1	25	2.1	9.13	10.21
FOS_68	1.0	4	10.0	10.0	2.2	25	3.2	10.47	15.56
FOS 70	1.9	4	10.0	10.0	2.6	25	3.6 3.9	4.67 5.52	7.30 8.89
FOS_71	3.1	4	10.0	10.0	3.3	25	4.3	2.21	3.67
FOS_72	1.5	4	10.0	10.0	2.6	25	3.6	5.78	9.00
FOS_73 FOS_74	1.0	4	10.0	10.0	3.6	25	4.6	6.32 8.17	10.73
FOS_75	1.0	4	10.0	10.0	2.6	25	3.6	8.92	13.91
FOS_76	1.0	4	10.0	10.0	2.5	25	3.5	9.11	14.12
FOS_77	1.0	4	10.0	10.0	2.9	25	3.9	7.94 6.82	12.77
FOS_79	1.8	4	10.0	10.0	1.0	25	2.0	12.47	13.73
FOS_80	1.0	4	10.0	10.0	1.0	25	2.0	22.68	24.69
FOS_81 FOS_82	1.0	4	10.0	10.0	2.2	25	3.2	14.34	15.35
FOS_83	1.0	4	10.0	10.0	2.9	25	3.9	7.80	12.61
FOS_84	1.0	4	10.0	10.0	2.7	25	3.7	8.35	13.25
FOS_85 FOS_86	1.8	4	10.0	10.0	1.0	25	2.0	22.92	24.82
FOS_87	1.0	4	10.0	10.0	1.0	25	2.0	22.92	24.82
FOS_88	2.3	4	10.0	10.0	1.0	25	2.0	9.55	10.45
FOS_90	1.0	4	10.0	10.0	1.1	25	2.1	17.32	21.36
FOS_91	1.0	4	10.0	10.0	1.2	25	2.2	19.74	22.97
FOS_92	1.0	4 A	10.0	10.0	1.1	25	2.1	20.72	23.57
FOS_94	1.0	4	10.0	10.0	1.4	25	2.4	20.84	23.64
FOS_95	1.0	4	10.0	10.0	0.6	25	1.6	37.54	30.82
FOS_96	1.0	4 A	10.0	10.0	0.9	25	1.9 1 A	26.27	26.51
FOS_98	1.0	4	10.0	10.0	0.4	25	1.4	57.52	35.49
FOS_99	1.0	4	10.0	10.0	0.8	25	1.8	27.55	27.09
FOS_100 FOS_101	1.0	4	10.0	10.0	1.2	25	2.2	19.68	22.93
FOS_101	1.4	4	10.0	10.0	0.6	25	1.6	27.00	22.32
FOS_103	3.3	4	10.0	10.0	0.1	25	1.1	93.01	13.92
FOS 105	7.2	4	10.0	10.0	0.4	25	1.4	8.55	5.03
FOS_106	1.0	4	10.0	10.0	2.1	25	3.1	11.15	16.25
FOS_107	1.0	4	10.0	10.0	1.7	25	2.7	13.33	18.25
FOS 109	1.0	4	10.0 10.0	10.0	0.8	25	1.8 1.2	27.79 96.80	27.20 40.13
FOS_110	0.9	4	10.0	10.0	0.3	25	1.3	88.70	44.41
FOS_111	1.1	4	10.0	10.0	0.4	25	1.4	57.15	32.09
FOS 112	1.0	4	10.0	10.0	1.2	25	2.2	18.54 18.42	22.19
FOS_114	1.0	4	10.0	10.0	0.7	25	1.7	32.02	28.93
FOS_115	1.0	4	10.0	10.0	0.6	25	1.6	35.39	30.13
FOS 117	1.0	4	10.0	10.0	1.0	25	2.0	24.08	25.43 17.42
FOS_118	2.3	4	10.0	10.0	2.0	25	3.0	5.13	7.34
FOS_119	2.0	4	10.0	10.0	1.5	25	2.5	7.51	9.81
FOS_120 FOS_121	1.0	4	10.0	10.0	1.5	25	2.5	15.67 35.54	30.18
FOS_122	3.0	4	10.0	10.0	1.0	25	2.0	7.66	8.23
FOS_123	1.9	4	10.0	10.0	0.7	25	1.7	15.72	14.57
FOS_124 FOS_125	2.1	4	10.0	10.0	2.1	25	3.1 2.5	5.12	7.52 19.83
FOS_126	1.0	4	10.0	10.0	1.7	25	2.7	13.49	18.39
FOS_127	1.0	4	10.0	10.0	1.5	25	2.5	14.94	19.58
FOS_128 FOS_129	1.0	4	10.0	10.0	1.0	25	2.8	6.75	9.39
FOS_130	1.0	4	10.0	10.0	2.0	25	3.0	11.59	16.67
FOS_131	1.0	4	10.0	10.0	2.0	25	3.0	11.24	16.33
FOS_132	1.0	4	10.0	10.0	1.5	25	2.5	15.17	4.76
FOS_134	3.6	4	10.0	10.0	1.0	25	2.0	6.70	7.06
FOS_135	2.4	4 A	10.0	10.0	1.8	25	2.8	5.19	7.27
FOS_130	1.0	4	10.0	10.0	1.5	25	2.5	15.47	20.00
FOS_138	2.6	4	10.0	10.0	0.9	25	1.9	9.39	9.78
FOS_139	1.0	4	10.0	10.0	0.4	25	1.4	54.92	35.02
FOS_140	1.0	4	10.0	10.0	1.1	25	2.5	15.78	20.24
FOS_142	1.0	4	10.0	10.0	1.3	25	2.3	17.15	21.24
FOS_143	2.6	4 A	10.0	10.0	0.5	25	1.5	17.34 9.99	12.55
FOS_145	1.0	4	10.0	10.0	1.5	25	2.5	<u>15.40</u>	19.95
FOS_146	1.0	4	10.0	10.0	1.2	25	2.2	19.82	23.02
FOS_147	1.1	4 A	10.0	10.0	1.0	25	2.0	19.77	21.53
FOS_148	1.9	4	10.0	10.0	3.0	25	4.0	7.56	12.31
FOS_150	1.0	4	10.0	10.0	2.7	25	3.7	8.61	13.56
FOS_151	1.0	4	10.0	10.0	1.4	25	2.4	16.20	20.55
FOS_152	1.0	4	10.0	10.0	1.4	25	2.4	21.34	23.93
FOS_154	2.4	4	10.0	10.0	0.9	25	1.9	10.69	10.91
FOS_155	1.7	4	10.0	10.0	0.7	25	1.7	19.59	17.18
FOS_156 FOS_157	3.8 1.1	4	10.0	10.0	0.5	25	1.5	24.12	23.68
FOS_158	1.7	4	10.0	10.0	1.0	25	2.0	13.85	14.71
FOS_159 FOS_160	1.0	4	10.0	10.0	1.1	25 25	2.1	21.47	24.01

Calc	ulated	FoS of	Natural Pe	at Slopes	for Derri	nlough	Wind Farm -	Drained A	nalysis
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety f	or Load Condition
	α (deg)	c' (kPa)	Peat γ (kN/m³)	γ _w (kN/m³)	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
								100% Water	100% Water
FOS 162	1.0	4	10.0	10.0	0.6	25	1.6 2.1	35.94 20.84	30.31 23.63
FOS_163	1.0	4	10.0	10.0	2.3	25	3.3	9.89	14.96
FOS_164	1.0	4	10.0	10.0	1.5	25	2.5	15.67	20.15
FOS_165	7.0	4	10.0	10.0	0.2	25	1.2	20.54	6.15
FOS_166	1.0	4	10.0	10.0	1.0	25	2.0	21.88	24.24
FOS_167	1.0	4	10.0	10.0	1.9	25	2.9	12.20	17.24
FOS 169	1.0	4	10.0	10.0	1.4	25	2.4	16.45	20.74
FOS_170	2.0	4	10.0	10.0	0.9	25	1.9	12.67	13.01
FOS_171	1.0	4	10.0	10.0	0.9	25	1.9	24.46	25.62
FOS_172	1.8	4	10.0	10.0	1.0	25	2.0	12.85	13.72
FOS 174	4.0	4	10.0	10.0	1.1	25	2.1	4.72	5.42
FOS_175	3.0	4	10.0	10.0	1.4	25	2.4	5.46	6.91
FOS_176	1.0	4	10.0	10.0	1.5	25	2.5	15.78	20.24
FOS_177	1.0	4	10.0	10.0	1./	25	2.7	13.70	18.56
FOS_178	1.0	4	10.0	10.0	1.9	25	2.9	12.26	17.29
FOS_180	2.5	4	10.0	10.0	0.6	25	1.6	16.51	12.89
FOS_181	1.0	4	10.0	10.0	0.5	25	1.5	48.08	33.61
FOS_182	1.3	4	10.0	10.0	0.5	25	1.5	38.23	25.89
FOS 184	2.0	4	10.0	10.0	0.4	25	1.4	02.82	30.37
FOS_185	1.5	4	10.0	10.0	0.2	25	1.2	90.88	28.50
FOS_186	1.0	4	10.0	10.0	0.7	25	1.7	34.15	29.70
FOS_187	2.7	4	10.0	10.0	0.9	25	1.9	9.65	9.79
FOS 189	1.0	4	10.0	10.0	0.8	25	1.8	28.84	27.66
FOS_190	1.0	4	10.0	10.0	1.3	25	2.3	17.75	21.66
FOS_191	3.9	4	10.0	10.0	1.6	25	2.6	3.68	4.90
FOS_192	1.0	4	10.0	10.0	0.7	25	1.7	32.78	29.21
FOS_193	2.7	4	10.0	10.0	0.7	25	1.7	11.81	10.71
FOS_194 FOS_195	1.0	4	10.0	10.0	1.3	25	2.3	17.27	21.33
FOS_196	1.0	4	10.0	10.0	1.6	25	2.6	14.61	19.32
FOS_197	1.9	4	10.0	10.0	2.0	25	3.0	6.14	8.82
FOS_198	2.2	4	10.0	10.0	1.2	25	2.2	8.57	10.23
FOS_199	1.0	4	10.0	10.0	1.6	25	2.6	14.33	19.09
FOS_200	1.0	4	10.0	10.0	1.2	25	2.2	18.36	22.08
FOS_202	1.0	4	10.0	10.0	0.4	25	1.4	54.27	34.90
FOS_203	0.7	4	10.0	10.0	0.6	25	1.6	60.07	46.43
FOS_204	0.7	4	10.0	10.0	0.9	25	1.9	38.99	38.92
FOS_205	1.0	4	10.0	10.0	0.9	25	1.9	27.49	27.46
FOS_207	3.7	4	10.0	10.0	0.7	25	1.7	8.77	7.91
FOS_208	1.0	4	10.0	10.0	0.6	25	1.6	37.25	30.73
FOS_209	1.0	4	10.0	10.0	1.8	25	2.8	12.81	17.80
FOS_210	1.0	4	10.0	10.0	3.2	25	4.2	7.21	11.87
FOS 212	1.8	4	10.0	10.0	1.0	25	2.0	12.71	13.51
FOS_213	1.0	4	10.0	10.0	1.1	25	2.1	20.46	23.41
FOS_214	1.0	4	10.0	10.0	1.1	25	2.1	21.45	24.00
FOS_215	1.9	4	10.0	10.0	1.0	25	2.0	11.56	12.62
FOS_216	1.8	4	10.0	10.0	1.2	25	2.2	11.16	12.96
FOS_218	1.0	4	10.0	10.0	2.4	25	3.4	9.45	14.49
FOS_219	1.0	4	10.0	10.0	1.7	25	2.7	14.24	19.21
FOS_220	1.2	4	10.0	10.0	0.9	25	1.9	21.56	21.90
FOS_221	1.0	4	10.0	10.0	0.6	25	1.6	39.18	31.32
FOS 222	1.0	4	10.0	10.0	1.9	25	2.9	32.83 11.91	29.23
FOS_224	3.0	4	10.0	10.0	2.2	25	3.2	3.45	5.12
FOS_225	2.4	4	10.0	10.0	1.3	25	2.3	7.24	8.91
FOS_226	1.0	4	10.0	10.0	2.7	25	3.7	8.63	13.58
FOS 222	1.0	4 4	10.0	10.0	1.9	25	2.9	11.99	17.05
FOS_229	1.0	4	10.0	10.0	0.5	25	1.5	42.87	32.34
FOS_230	1.0	4	10.0	10.0	0.2	25	1.2	116.24	41.46
FOS_231	1.0	4	10.0	10.0	0.1	25	1.1	164.09	43.55
FUS_232	1.0	4	10.0	10.0	0.2	25	1.2	116.72	41.49
FOS 234	9.1	4	10.0	10.0	1.3	25	2.3	1.90	23.02
FOS_235	1.0	4	10.0	10.0	0.9	25	1.9	25.78	26.27
FOS_236	17.4	4	10.0	10.0	0.1	25	1.1	10.08	2.53
FOS_237	1.0	4	10.0	10.0	0.5	25	1.5	43.08	32.40
FOS 238	2./	4	10.0	10.0	0.8	25	1.8	20,61	10.20
FOS_240	2.3	4	10.0	10.0	1.4	25	2.4	7.35	9.18
FOS_241	3.1	4	10.0	10.0	1.0	25	2.0	7.59	8.12
FOS_242	2.2	4	10.0	10.0	1.1	25	2.1	9.45	10.79
FOS_243	2.1	4	10.0	10.0	1.0	25	2.0	11.11	12.03
FOS 244	3.2	4	10.0	10.0	1.1	25	2.1	6.74 7.40	7.51 8.25
FOS_246	5.0	4	10.0	10.0	1.7	25	2.7	2.66	3.63
FOS_247	3.7	4	10.0	10.0	1.3	25	2.3	4.87	5.92
FOS_248	0.5	4	10.0	10.0	0.9	25	1.9	46.86	49.40
FUS_249	0.5	4	10.0	10.0	1.2	25	2.2	41.83	49.33
FOS 251	1.0	4	10.0	10.0	0.8	25	1.8	27.08	26.88
FOS_252	1.0	4	10.0	10.0	1.3	25	2.3	17.52	21.50
FOS_253	0.6	4	10.0	10.0	1.5	25	2.5	26.79	34.75
FOS_254	0.7	4	10.0	10.0	1.4	25	2.4	21.24	27.22

Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm						Wind Farm	n - Drained Analysis		
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of Peat	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety f	or Load Condition
	α (deg)	c' (kPa)	γ (kN/m ³)	$\gamma_{\rm w}(kN/m^3)$	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
EQS 256	10	4	10.0	10.0	16	25	2.6	100% Water 13.95	100% Water 18,78
FOS_257	1.0	4	10.0	10.0	1.5	25	2.5	16.02	20.64
FOS_258	0.8	4	10.0	10.0	1.1	25	2.1	26.19	29.59
FOS_259 FOS_260	0.7	4	10.0	10.0	1.0	25	2.0	31.86 21.83	35.28 23.21
FOS_261	1.4	4	10.0	10.0	0.6	25	1.6	25.75	21.92
FOS_262	1.2	4	10.0	10.0	0.5	25	1.5	40.72	28.11
FOS 263	0.9	4	10.0	10.0	0.4	25	1.4	72.38	42.21
FOS_265	1.0	4	10.0	10.0	0.6	25	1.6	30.81	26.27
FOS_266	1.0	4	10.0	10.0	1.0	25	2.0	22.14	24.39
FOS_267	0.9	4	10.0	10.0	0.7	25	1.7	36.76	33.47
FOS_269	1.0	4	10.0	10.0	2.6	25	3.6	8.74	13.71
FOS_270	2.5	4	10.0	10.0	1.5	25	2.5	5.99	7.82
FOS_271	2.4	4	10.0	10.0	0.7	25	1.7	14.51 50 14	12.45
FOS_272	1.0	4	10.0	10.0	0.8	25	1.5	27.03	26.86
FOS_274	1.0	4	10.0	10.0	1.3	25	2.3	17.65	21.60
FOS_275	1.0	4	10.0	10.0	0.5	25	1.5	48.62	33.73
FOS_277	3.1	4	10.0	10.0	1.4	25	2.4	5.25	6.65
FOS_278	1.0	4	10.0	10.0	1.0	25	2.0	23.41	25.08
FOS_279	3.1	4	10.0	10.0	1.8	25	2.8	4.13	5.71
FOS_280 FOS_281	4.0	4	10.0	10.0	1.5	25	2.3	4.55	5.48
FOS_282	4.1	4	10.0	10.0	1.0	25	2.0	5.58	6.07
FOS_283	1.9	4	10.0	10.0	1.2	25	2.2	10.34	12.08
FOS_284	2.4	4	10.0	10.0	0.9	25	2.1 1.9	8.77 9.52	9.89
FOS_286	1.4	4	10.0	10.0	0.5	25	1.5	32.77	23.29
FOS_287	1.0	4	10.0	10.0	0.1	25	1.1	219.57	44.95
FOS 288	1.0	4	10.0	10.0	0.1	25	1.1	342.65 38141.90	46.53 24.76
FOS_290	1.0	4	10.0	10.0	0.0	25	1.0	5730.74	49.44
FOS_291	1.0	4	10.0	10.0	0.0	25	1.0	57307.42	49.62
FOS_292	1.0	4	10.0	10.0	0.0	25	1.0	38204.94	49.61
FOS_293 FOS_294	3.6	4	10.0	10.0	0.0	25	1.0	1028.13	49.60
FOS_295	1.0	4	10.0	10.0	0.0	25	1.0	229229.67	49.63
FOS_296	1.0	4	10.0	10.0	0.0	25	1.0	7163.43	49.48
FOS 297	5.4	4	10.0	10.0	0.0	25	1.0	196.92	9.15
FOS_299	1.0	4	10.0	10.0	0.0	25	1.0	22922.97	49.59
FOS_300	3.1	4	10.0	10.0	0.0	25	1.0	10612.87	16.05
FOS 301	23.2	4	10.0	10.0	0.0	25	1.0	197.14	2.18
FOS_303	1.0	4	10.0	10.0	0.1	25	1.1	<u>1</u> 67.44	43.66
FOS_304	1.0	4	10.0	10.0	0.1	25	1.1	445.11	47.21
FOS_305	2.3	4	10.0	10.0	0.0	25	1.0	273.66	20.91
FOS_307	0.6	4	10.0	10.0	0.4	25	1.4	96.14	61.18
FOS_308	0.5	4	10.0	10.0	0.8	25	1.8	63.02	60.38
FOS_309	0.5	4	10.0	10.0	0.4	25	1.4	121.57	76.73
FOS_310 FOS_311	2.3	4	10.0	10.0	0.5	25	1.5 1.4	46.89 22.11	33.34 14.66
FOS_312	1.6	4	10.0	10.0	0.3	25	1.3	43.46	23.29
FOS_313	1.0	4	10.0	10.0	0.5	25	1.5	48.89	33.79
FOS 314	1.0	4	10.0	10.0	0.3	25	1.3	78.05	38.37 34.34
FOS_316	3.1	4	10.0	10.0	1.0	25	2.0	7.34	7.98
FOS_317	6.7	4	10.0	10.0	1.2	25	2.2	2.90	3.39
FOS_318 FOS_319	3.1	4 A	10.0	10.0	1.8	25	2.8	4.17	5.77
FOS_320	3.9	4	10.0	10.0	0.6	25	1.6	10.41	8.07
FOS_321	1.0	4	10.0	10.0	1.3	25	2.3	17.31	21.35
FOS_322	1.0	4	10.0	10.0	0.4	25	1.4	56.31	35.28
FOS_324	1.0	4 4	10.0	10.0	0.5	25	1.5	31.35	28.67
FOS_325	1.0	4	10.0	10.0	0.7	25	1.7	32.71	29.19
FOS_326	6.5	4	10.0	10.0	1.7	25	2.7	2.07	2.81
FOS_327 FOS_328	7.5	4	10.0	10.0	2.2	25	3.2	10.38	15.47
FOS_329	11.0	4	10.0	10.0	1.7	25	2.7	1.29	1.71
FOS_330	15.0	4	10.0	10.0	0.8	25	1.8	1.89	1.81
FOS 331	2.3	4	10.0	10.0	0.6	25	1.6	16.66 26.41	13.33
FOS_333	1.0	4	10.0	10.0	0.9	25	1.9	24.19	25.49
FOS_334	1.0	4	10.0	10.0	0.7	25	1.7	31.12	28.58
FOS_335	1.2	4 A	10.0	10.0	0.6	25	1.6	32.12	25.90
FOS_337	1.0	4	10.0	10.0	0.9	25	1.9	22.41	22.30
FOS_338	1.0	4	10.0	10.0	1.0	25	2.0	23.71	25.24
FOS_339	2.8	4	10.0	10.0	0.8	25	1.8	10.46	9.93
FOS_340 FOS_341	1.0	4	10.0	10.0	0.7	25	1.7	32.11 26.47	28.96
FOS_342	3.1	4	10.0	10.0	0.9	25	1.9	7.94	8.22
FOS_343	1.0	4	10.0	10.0	0.4	25	1.4	53.06	34.66
FOS 344	1.0	4	10.0	10.0	1.0	25	2.0	24.02	25.40
FOS_346	1.0	4	10.0	10.0	0.9	25	1.9	25.62	26.20
FOS_347	1.0	4	10.0	10.0	1.1	25	2.1	21.63	24.10
FOS_348	1.0	4	10.0	10.0	1.2	25	2.2	19.45	22.79
FOS 350	1.0	4	10.0	10.0	1.0	25	2.0	18.99	24.55

Calc	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm						Drained Analysis		
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety f	or Load Condition
	α (deg)	c' (kPa)	Peat γ (kN/m³)	γ _w (kN/m³)	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
								100% Water	100% Water
FOS_351	1.0	4 A	10.0	10.0	1.4	25	2.4	16.41 14 87	20.71
FOS_353	1.0	4	10.0	10.0	1.4	25	2.5	15.81	20.26
FOS_354	1.0	4	10.0	10.0	1.4	25	2.4	15.84	20.29
FOS_355	1.0	4	10.0	10.0	1.6	25	2.6	14.49	19.22
FOS_356	1.8	4	10.0	10.0	1.4	25	2.4	8.90	11.25
FOS 358	1.0	4	10.0	10.0	1.3	25	2.3	18.06	21.87
FOS_359	2.5	4	10.0	10.0	0.7	25	1.7	13.97	12.10
FOS_360	1.0	4	10.0	10.0	0.1	25	1.1	208.01	44.71
FOS_361 FOS_362	1.2	4	10.0	10.0	0.5	25	1.5	41.94	28.37
FOS_363	2.2	4	10.0	10.0	0.9	25	1.9	12.23	12.25
FOS_364	1.0	4	10.0	10.0	0.9	25	1.9	26.75	26.73
FOS_365	1.0	4	10.0	10.0	0.8	25	1.8	29.05	27.74
FOS_366	1.0	4	10.0	10.0	0.9	25	1.9	22.55	24.61
FOS_368	1.9	4	10.0	10.0	1.7	25	2.7	7.22	9.80
FOS_369	1.0	4	10.0	10.0	1.0	25	2.0	23.94	25.36
FOS_370 FOS_371	1.0	4	10.0	10.0	0.6	25	1.6	35.44	30.14
FOS_372	1.4	4	10.0	10.0	0.5	25	1.5	30.13	22.63
FOS_373	1.0	4	10.0	10.0	2.7	25	3.7	8.48	13.40
FOS 275	6.3	4	10.0	10.0	0.4	25	1.4	8.98	5.62
FOS_376	3.6	4	10.0	10.0	0.2	25	1.2	22.94	10.78
FOS_377	1.0	4	10.0	10.0	1.1	25	2.1	20.17	23.23
FOS_378	1.6	4	10.0	10.0	0.5	25	1.5	26.47	20.10
FOS_379 FOS_380	1.0	4	10.0	10.0	0.5	25	1.5	46.65	33.28
FOS_381	1.0	4	10.0	10.0	0.2	25	1.2	148.08	42.98
FOS_382	1.0	4	10.0	10.0	0.7	25	1.7	34.68	29.88
FOS_383	1.0	4	10.0	10.0	0.8	25	1.8	27.85	27.23
FOS_384	1.0	4	10.0	10.0	2.4	25	3.4	9.75	14.81
FOS_386	1.0	4	10.0	10.0	1.2	25	2.2	18.36	22.08
FOS_387	1.0	4	10.0	10.0	0.9	25	1.9	25.76	26.26
FOS_388	8.4	4	10.0	10.0	0.7	25	1.7	33.82	3.10
FOS_390	1.0	4	10.0	10.0	0.5	25	1.5	50.36	34.11
FOS_391	1.0	4	10.0	10.0	0.9	25	1.9	25.63	26.20
FOS_392	1.0	4	10.0	10.0	0.8	25	1.8	28.65	27.57
FOS_394	0.7	4	10.0	10.0	1.4	25	2.3	21.68	27.54
FOS_395	1.0	4	10.0	10.0	1.6	25	2.6	14.62	19.33
FOS_396	1.0	4	10.0	10.0	1.1	25	2.1	20.86	23.65
FOS 398	1.0	4	10.0	10.0	0.4	25	1.4	60.69	36.03
FOS_399	1.5	4	10.0	10.0	0.8	25	1.8	19.22	18.12
FOS_400	1.6	4	10.0	10.0	0.5	25	1.5	27.01	20.24
FOS_401 FOS_402	1.0	4	10.0	10.0	0.7	25	1.7	32.15	28.98
FOS_403	1.0	4	10.0	10.0	0.1	25	1.1	248.89	45.45
FOS_404	4.3	4	10.0	10.0	0.9	25	1.9	6.02	6.12
FOS_405	2.9	4	10.0	10.0	1.6	25	2.6	4.82	6.46 27.35
FOS_407	1.9	4	10.0	10.0	0.7	25	1.7	17.23	15.41
FOS_408	3.1	4	10.0	10.0	0.3	25	1.3	26.52	12.37
FOS_409	2.1	4	10.0	10.0	0.5	25	1.5	20.63	15.36
FOS_410	1.0	4	10.0	10.0	0.0	25	1.0	24.20	25.49
FOS_412	1.0	4	10.0	10.0	1.0	25	2.0	23.93	25.35
FOS_413	1.8	4	10.0	10.0	0.6	25	1.6	21.26	17.05
FOS 414	1.1	4	10.0	10.0	0.1	25	1.1	189.05 51.25	41.03
FOS_416	2.5	4	10.0	10.0	0.5	25	1.5	18.06	13.30
FOS_417	1.0	4	10.0	10.0	0.9	25	1.9	24.60	25.69
FOS_418	3.3	4	10.0	10.0	0.7	25	1.7	10.19	9.00
FOS_420	4.9	4	10.0	10.0	0.8	25	1.8	6.05	5.73
FOS_421	4.1	4	10.0	10.0	0.6	25	1.6	8.80	7.44
FOS_422	4.3	4	10.0	10.0	0.7	25	1.7	7.21	6.64
FOS 423	5.5	4	10.0	10.0	0.6	25	1.6 1.9	7.22	5.69 7.91
FOS_425	1.3	4	10.0	10.0	1.3	25	2.3	12.91	16.04
FOS_426	1.0	4	10.0	10.0	2.6	25	3.6	8.71	13.67
FOS 427	1.0	4	10.0	10.0	1.6	25	2.6	14.03	18.84
FOS_429	1.0	4	10.0	10.0	1.5	25	2.5	14.95	19.60
FOS_430	1.0	4	10.0	10.0	1.1	25	2.1	20.11	23.19
FOS_431	1.0	4	10.0	10.0	1.2	25	2.2	19.76	22.98
FOS 432	1.0	4	10.0	10.0	1.1	25	2.1	20.54	23.46
FOS_434	0.6	4	10.0	10.0	0.9	25	1.9	44.53	45.64
FOS_435	0.9	4	10.0	10.0	0.7	25	1.7	36.97	33.55
FOS_436	0.9	4	10.0	10.0	0.4	25	1.4	61.01 9.91	40.19
FOS_438	4.9	4	10.0	10.0	0.9	25	1.9	4.95	5.19
FOS_439	1.7	4	10.0	10.0	0.5	25	1.5	25.70	19.44
FOS_440	1.0	4	10.0	10.0	0.1	25	1.1	207.64	44.70
FOS 441 FOS 442	4.3	4	10.0	10.0	0.3	25	2.3	4.23	37.54 5.08
FOS_443	2.8	4	10.0	10.0	0.7	25	1.7	12.38	10.66
FOS_444	1.4	4	10.0	10.0	0.6	25	1.6	28.77	22.86
EOS 445	10	4	10.0	10.0	0.9	25	19	26 21	26.85

Calc	Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm							 Drained Analysis 	
Turbine No./Waypoint	Slope	Design c'	Bulk unit weight of Peat	Unit weight of Water	Depth of In situ Peat	Friction Angle	Equivalent Total Depth of Peat (m)	Factor of Safety f	or Load Condition
	α (deg)	c' (kPa)	γ (kN/m³)	$\gamma_{\rm w} (kN/m^3)$	(m)	ø' (deg)	Condition (2)	Condition (1)	Condition (2)
FOS 446	0.9	4	10.0	10.0	0.6	25	1.6	40.31	33.42
FOS_447	1.3	4	10.0	10.0	0.6	25	1.6	28.11	23.91
FOS_448	1.0	4	10.0	10.0	0.7	25	1.7	32.99	29.29
FOS_449 FOS_450	1.0	4	10.0	10.0	0.5	25	2.1	44.30 21.28	32.71
FOS_451	3.4	4	10.0	10.0	0.7	25	1.7	9.52	8.54
FOS_452	1.0	4	10.0	10.0	0.5	25	1.5	49.93	34.02
FOS_453 FOS_454	1.0	4	10.0	10.0	0.4	25	1.4	54.35	34.91
FOS_455	1.0	4	10.0	10.0	0.4	25	1.4	57.34	35.46
FOS_456	2.3	4	10.0	10.0	0.5	25	1.5	18.45	13.82
FOS_457	1.0	4	10.0	10.0	1.0	25	2.0	22.53	24.60
FOS_458	1.0	4	10.0	10.0	0.8	25	1.5	27.93	27.26
FOS_460	1.0	4	10.0	10.0	1.0	25	2.0	23.47	25.11
FOS_461	2.1	4	10.0	10.0	1.2	25	2.2	9.13	10.72
FOS_463	2.6	4	10.0	10.0	1.4	25	2.4	7.14	8.49
FOS_464	1.0	4	10.0	10.0	1.1	25	2.1	20.19	23.25
FOS_465	2.3	4	10.0	10.0	1.1	25	2.1	9.45	10.52
FOS_466	4.1 9.6	4	10.0	10.0	1.0	25	2.0	2.43	2.60
FOS_468	3.8	4	10.0	10.0	1.0	25	2.0	6.06	6.56
FOS_469	1.0	4	10.0	10.0	0.8	25	1.8	29.89	28.09
FOS 470	1.0	4	10.0	10.0	0.4	25	1.4	55.28 46.57	35.09
FOS_472	1.0	4	10.0	10.0	0.4	25	1.4	61.89	36.22
FOS_473	3.0	4	10.0	10.0	0.2	25	1.2	45.62	14.04
FOS 474	3.5	4	10.0	10.0	0.0	25 25	1.0 1 1	275.39	13.89 15.68
FOS_476	2.4	4	10.0	10.0	0.2	25	1.2	46.31	17.12
FOS_477	1.0	4	10.0	10.0	0.1	25	1.1	391.85	46.89
FOS_478	2.7	4	10.0	10.0	0.1	25	1.1	105.73	16.74
FOS 480	4.9	4	10.0	10.0	0.4	25	1.4	3.43	2.95
FOS_481	14.1	4	10.0	10.0	1.0	25	2.0	1.76	1.81
FOS_482	12.5	4	10.0	10.0	1.2	25	2.2	1.62	1.84
FOS_483	7.5	4	10.0	10.0	0.8	25	2.0	2.98	2.78
FOS_485	2.5	4	10.0	10.0	0.5	25	1.5	17.53	12.97
FOS_486	2.5	4	10.0	10.0	0.4	25	1.4	22.73	14.30
FOS_487	2.5	4	10.0	10.0	0.4	25	1.4	21.34	14.03
FOS_489	1.0	4	10.0	10.0	0.5	25	1.5	43.14	32.41
FOS_490	1.0	4	10.0	10.0	0.6	25	1.6	37.34	30.76
FOS_491	1.8	4	10.0	10.0	0.7	25	1.7	18.88	16.60
FOS 493	1.0	4	10.0	10.0	0.8	25	1.8	27.81	27.21
FOS_494	1.6	4	10.0	10.0	1.0	25	2.0	14.71	15.70
FOS_495	2.5	4	10.0	10.0	1.3	25	2.3	7.11	8.64
FOS_496 FOS_497	4.2	4	10.0	10.0	1.6	25	2.6	3.55	4.66
FOS_498	2.0	4	10.0	10.0	2.0	25	3.0	5.74	8.28
FOS_499	1.3	4	10.0	10.0	2.0	25	3.0	8.70	12.56
FOS_500 FOS_501	2.9	4	10.0	10.0	1./	25	2.7	4.69	6.35
FOS_502	2.1	4	10.0	10.0	0.9	25	1.9	12.63	12.62
FOS_503	1.0	4	10.0	10.0	0.9	25	1.9	25.63	26.20
FOS_504	1.0	4	10.0	10.0	0.9	25	1.9	24.63	25.71
FOS_506	1.0	4	10.0	10.0	1.0	25	2.0	23.64	25.24
FOS_507	3.4	4	10.0	10.0	0.8	25	1.8	8.35	8.11
FOS_508	1.0	4	10.0	10.0	0.8	25	1.8	28.73	27.61
FOS_517 FOS_518	1.0	4	10.0	10.0	0.1	25	1.1	137.76	45.89
FOS_519	1.0	4	10.0	10.0	0.3	25	1.3	69.11	37.27
FOS_520	1.0	4	10.0	10.0	0.5	25	1.5	47.62	33.51
FOS 522	1.0	4	10.0	10.0	0.6	25	1.6	37.71 43.40	30.87
FOS_523	1.0	4	10.0	10.0	0.3	25	1.3	77.31	38.29
FOS_524	2.7	4	10.0	10.0	0.5	25	1.5	17.06	12.13
FOS_525	2.6	4	10.0	10.0	0.9	25	1.9	10.05 8 10	10.10
FOS_527	1.0	4	10.0	10.0	1.5	25	2.3	12.76	17.75
FOS_528	3.7	4	10.0	10.0	1.8	25	2.8	3.45	4.81
FOS_529	3.6	4	10.0	10.0	1.5	25	2.5	4.23	5.50
FOS 531	1.0	4	10.0	10.0	1.3	25	2.3	3.08	4.43
FOS_532	1.0	4	10.0	10.0	0.8	25	1.8	27.65	27.14
FOS_533	1.0	4	10.0	10.0	0.8	25	1.8	27.31	26.99
FOS 525	1.0	4 4	10.0	10.0	0.9	25	1.9	26.55	26.64
FOS_536	1.0	4	10.0	10.0	1.0	25	2.0	23.09	24.91
FOS_537	5.5	4	10.0	10.0	1.1	25	2.1	3.67	4.23
FOS_538	4.1	4	10.0	10.0	0.8	25	1.8	7.30	6.83
FOS_540	1.0	4	10.0	10.0	1.2	25	2.2	18.52	22.18
FOS_541	1.0	4	10.0	10.0	1.0	25	2.0	22.49	24.58
FOS_542	3.3	4	10.0	10.0	2.3	25	3.3	2.99	4.52
FUS_543 FOS 544	3.2	4	10.0	10.0	2.8	25	3.8 1.2	8.24 36.69	13.12
FOS_545	1.0	4	10.0	10.0	2.9	25	3.9	7.98	12.82
FOS_546	1.0	4	10.0	10.0	3.0	25	4.0	7.63	12.39
FOS_547 FOS_548	1.0	4	10.0	10.0	1.3	25 25	2.3	17.20 36.69	21.28

Calculated FoS of Natural Peat Slopes for Derrinlough Wind Farm - Drained Analysis									
Turbine	Slope	Design c'	Bulk unit weight	Unit weight	Depth of In	Friction	Equivalent Total	Factor of Safety for Load Condition	
NO./ Waypoint			Peat	or water situ reat Angle Depth of Peat (m)		Depth of Peat (III)			
	α (deg)	c' (kPa)	γ (kN/m³)	γ _w (kN/m³)	(kN/m³) (m)		Condition (2)	Condition (1) Condition (2)	
								100% Water	100% Water
FOS_549	1.0	4	10.0	10.0	1.1	25	2.1	21.34	23.93
FOS_550	5.1	4	10.0	10.0	1.3	25	2.3	3.58	4.29
FOS_551	2.0	4	10.0	10.0	0.1	25	1.1	89.33	21.95
FOS_552	1.0	4	10.0	10.0	0.4	25	1.4	59.25	35.79
FOS_553	1.0	4	10.0	10.0	0.2	25	1.2	98.85	40.29
FOS_554	1.0	4	10.0	10.0	0.5	25	1.5	46.09	33.15
FOS_555	1.0	4	10.0	10.0	1.0	25	2.0	23.68	25.22
FOS_556	11.5	4	10.0	10.0	1.3	25	2.3	1.53	1.85
FOS_557	1.0	4	10.0	10.0	1.0	25	2.0	22.46	24.56
FOS_558	1.5	4	10.0	10.0	0.8	25	1.8	19.29	18.15

1.08 229229.67 796.36 1.71 76.73 19.86 Minimum = Maximum = Average =

Notes:

 Notes:

 (1) Assuming a bulk unit weight of peat of 10 (kN/m³)

 (2) Assuming a surcharge equivalent to fill depth of 1.0m.

 (3) Slope inclination (β) based on site readings and topographical survey data.

 (4) FoS is based on slope inclination and shear test results obtained from published data.

 (5) Peat depths based on probes carried out by FT, HES and a GPR survey carried out by BnM.

 (6) For load conditions see Report text.

 (7) Minimum acceptable factor of safety required of 1.3 for first-time failures based on BS: 6031:1981 Code of practice for Earthworks.

Appendix D

Methodology for Peat Stability Risk Assessment









Methodology for Peat Stability Risk Assessment

A peat stability risk assessment was carried out for each of the main infrastructure elements at the proposed wind farm development. This approach takes into account guidelines for geotechnical/peat stability risk assessments as given in PLHRAG (2017) and MacCulloch (2005). The degree of risk is determined as a Risk Rating (R), which is the product of probability (P) and impact (I). How these factors are determined and applied in the analysis is described below.

The main approaches for assessing peat stability include the following:

- (a) Geomorphological
- (b) Qualitative (judgement)
- (c) Index/Probabilistic (probability)
- (d) Deterministic (factor of safety)

Approaches (a) to (c) listed above would be considered subjective and do not provide a definitive indication of stability; in addition, a high level of judgement/experience is required which makes it difficult to relate the findings to real conditions. FT apply a more objective approach, the deterministic approach. As part of FT's deterministic approach, a qualitative risk assessment is also carried out taking into account qualitative factors, which cannot necessarily be quantified.

Probability

The likelihood of a peat failure occurring was assessed based on the results of both the quantitative results of stability calculations (deterministic approach using factors of safety) and the assessment of the severity of several qualitative factors which cannot be reasonably included in a stability calculation but nevertheless may affect the occurrence of peat instability.

The qualitative factors used in the risk assessment are outlined in Table A and have been compiled based on FT's experience of assessments and construction in peat land sites and peat failures throughout Ireland and the UK.

Qualitative Factor	Type of Feature/Indicator for each Qualitative Factor ⁽¹⁾	Explanation/Description of Qualitative Factor		
	No	Based on site walkover observations. Sub peat water flow generally occurs in the form of natural piping at the base of peat. Where there is a constriction or blockage in natural		
Evidence of sub peat	Possibly			
water flow	Probably	 pipes a build-up of water can occur at the base of the peat causing a reduction in effective stress at the base of the peat resulting in failure; this is particularly critical during periods of intense rainfall. Based on site walkover observations. The presence of surface water flow indicates if peat in an area is well drained or saturated and if any additional loading from the ponding of surface water onto the peat is likely. 		
	Yes			
	Dry			
Evidence of surface water flow	Localised/Flowing in drains			
	Ponded in drains			

 Table A:
 Qualitative Factors used to Assess Potential for Peat Failure

Qualitative Factor	Type of Feature/Indicator for each Qualitative Factor ⁽¹⁾	Explanation/Description of Qualitative Factor		
	Springs/surface water			
	No	Based on site walkover observations. The presence of clustering of relict		
Evidence of previous	In general area			
failures/slips	On site	pre-existing site conditions predispose a site to failure.		
	Within 500m of location			
	Grass/Crops	Based on site walkover observations.		
Type of vegetation	Improved Grass/Dry Heather	The type of vegetation present indicates if peat in an area is well drained caturated ate Vegetation		
Type of vegetation	Wet Grassland/Juncus (Rushes)	that indicates wetter ground may also indicate softer underlying peat		
	Wetlands Sphagnum (Peat moss)	deposits.		
	Concave			
General slope characteristics	Planar to concave	Based on site walkover observations. Slope morphology in the area of the infrastructure location is an important		
upslope/downslope from infrastructure location	Planar to convex	factor. A number of recorded peat failures have occurred in close		
	Convex			
Evidence of very	No	Based on inspection of exposures in general area from site walkover. Several reported peat failures identify the presence of a weak layer at the base of the peat along which shear failure has occurred.		
peat	Yes			
5 iluna af	No	Based on site walkover observations. Mechanically cut peat typically cut using a 'sausage' machine to extract peat for harvesting. Areas which have been cut in this manner have been linked to peat instability. The mechanical cuts can notably reduce the intrinsic strength of the peat and also allow ingress of rainfall/surface water.		
mechanically cut peat	Yes			
	No	Based on site walkover observations. Quaking/buoyant peat is indicative of highly saturated peat, which would		
Evidence of quaking or buoyant peat	Yes	generally be considered to have a low strength. Quaking peat is a feature on sites that have been previously linked with peat instability.		

Qualitative Factor	Type of Feature/Indicator for each Qualitative Factor ⁽¹⁾	Explanation/Description of Qualitative Factor		
	No	Based on site walkover observations. Bog pools are generally an indicator of areas of weak, saturated peat. Commonly where there are open		
Evidence of bog pools	Yes	be interconnected, with the result that there may be sub-surface bodies of water. The presence of bog pools have been previously linked with peat instability.		
Other	Varies	In addition to the above features/ indicators and based on site recordings the following are some of the features which may be identified: Excessively deep peat, weak peat, overly steep slope angles, etc.		

Note (1) The list of features/indicators for each qualitative factor are given in increasing order of probability of leading to peat instability/failure.

It should be noted that the presence of one of the qualitative factors alone from Table A is unlikely to lead to peat instability/failure. Peat instability/failure at a site is generally the combination of a number of these factors occurring at the same time at a particular location. The probability rating assigned to the quantitative and qualitative factors is judged on a 5-point scale from 1 (indicating negligible or no probability of failure) to 5 (indicating a very likely failure), as outlined in Table B.

Table B: Probability Scale

Scale	Factor of Safety	Probability
1	1.30 or greater	Negligible/None
2	1.29 to 1.20	Unlikely
3	1.19 to 1.11	Likely
4	1.01 to 1.10	Probable
5	≤1.0	Very Likely

Scale	Likelihood of Qualitative Factor leading to Peat Failure	Probability of Failure
1	Negligible/None	Least
2	Unlikely	
3	Probable	
4	Likely	
5	Very Likely	Greatest

Impact

The severity of the risk is also assessed qualitatively in terms of impact. The impact of a peat failure on the environment within and beyond the immediate wind farm site is assessed based on the potential travel distance of a peat failure. Where a peat failure enters a watercourse, it can travel a considerable distance downstream. Therefore, the proximity of a potential peat failure to a drainage course is a significant indicator of the likely potential impact.

The risk is determined based on the combination of hazard and impact. A qualitative scale has been derived for the impact of the hazard based on distance of infrastructure element to a watercourse (Table C).

The location of watercourses is based on topographic maps and supplemented by site observations from walkover survey. Note that not all watercourses are shown on maps.

Scale	Criteria	Impact				
1	Proposed infrastructure element greater than 150m of watercourse	Negligible/None				
2	Proposed infrastructure element within 150 to 101m of watercourse	Low				
3	Proposed infrastructure element within 100 to 51m of watercourse	Medium				
4	Proposed infrastructure element within 50 m of watercourse	High				
5	Proposed infrastructure element within 50 m of watercourse, Extremely High					

Table C: Impact Scale

Risk Rating

The degree of risk is determined as the product of probability (P) and impact (I), which gives the Risk Rating (R) as follows:

The Risk Rating is calculated from: $R = P \times I$

Due to the 5-point scales used to assess Probability and Impact, the Risk Rating can range from 1 to 25 as shown in Table D.

			Proba	bility			Risk	Rating and Control Measures
		1	2	3	4	5	17 to 25	High: avoid working in area or significant control measures required
	5	5	10	15	20	25	11 to 16	Medium: notable control measures required
npact	4	4	8	12	16	20	5 to 10	Low: only routine control measures required
I	3	3	6	9	12	15	1 to 4	Negligible: none or only routine control measures required
	2	2	4	6	8	10		
	1	1	2	3	4	5		

Table D: Qualitative Risk Rating

The risk rating is calculated individually for each contributory factor. Control measures are required to reduce the risk to at least a 'Low' risk rating. The control measures in response to the qualitative risk ratings are included in the peat stability risk registers for each main infrastructure element in Appendix C.

The risk rating is calculated individually for each contributory factor. Control measures are required to reduce the risk to at least a 'Tolerable' risk rating.

Appendix E

Ground Investigation (January 2019): Trial Pit Logs, Photographs and Laboratory Test Results











Photo 1 Trial pit TP1



Photo 2 Trial pit TP2



Photo 3 Excavated arisings from trial pit TP2



Photo 4 Trial pit TP3



Photo 5 Trial pit TP4



Photo 6 Trial pit TP5



Photo 7 Trial pit TP6



Photo 8 Excavated arisings from trial pit TP6



Photo 9 Trial pit TP7



Photo 10 Trial pit TP8



Photo 11 Trial pit TP9



Photo 12 Trial pit TP10


Photo 13 Trial pit TP11



Photo 14 Trial pit TP12



Photo 15 Trial pit TP13



Photo 16 Trial pit TP14



Photo 17 Excavated arisings from trial pit TP14



Photo 18 Trial pit TP15

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		Sir Ba	ngletons Lane Ema agenalstown Co. Carlow www.	il: info@ftco.ie Web: fehilytimoney.ie			Irial Pi	t Log		TP1	
Т	IMONEY	R21	XA66 Ireland				1			Sheet 1 of 1	
Project	Derrinlo	ugh W	ind Farm		Projec	t No.	Coords (E,N):	607659.00	714995.00	Date	
iname.		•			1901		Level:		171.60	29/01/2019)
Location	County C	ffaly					Dimensions (m):			Scale 1.25	
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	Samp	les & In S	Situ Testina				2.30			GK	
Wate Strike	Depth	Туре	Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum Descr	ription		
	2.10 - 2.20	В	Sample B1	0.60 1 2.10 1 2.30 1	171.00		Spongy and firm Firm, locally soft sandy Silt/Clay v Cobbles and bou Doubles and bou boulder content. and angular. Ma	and locally stiff, c ith low boulder ar ilders are sub-rou gravelly Silt/Clay Cobbles and boul terial saturated up End of Pit at 2.	s Peat	elly t. Ir.	2 4
Remark	s: Groundwat Excavator	er - stea unable t	ady flow of groundwa o excavate deeper d	ter at 1.9m bg ue to presenc	l e of lar	ge boulde	rs			Plant Used: 12tN Tracked	
										Excavator	

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Project		Derrinlo	Jah Wi	ind Farm		Projec	t No.	Coords (E,N):	607714.00	715008.00	Date	
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Client:		Bord na M	lóna					Depth			Logge	d
								3.20]	GK	
Water Strike		Depth	Type	Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum Descr	iption		
	1.	50 - 1.70	В	Sample B2	0.60 1 2.10 1 3.20 1	I64.57 I63.07		Spongy and firm Firm, locally sof with low cobble are sub-rounded boulder content and angular. Ma	t and locally stiff, si and boulder conter d and angular.	andy gravelly S nt. Cobbles and with high cobbl ders are sub-ro on excavation	ilt/Clay d boulders	
Remark	s:	Groundwate Trial pit terr	er - stea ninated	dy flow of groundward due to instability of	ater at 2m bgl excavation fac	es					Plant Used 12tN Tracke Excavator	1: ed

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							1901		Level:			195.0	5 29/01/2 Soci	019
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Client:	E	Bord na N	/lóna						Depth				Logge	ed
		Sampl	es & In S	itu Testina					3.70				GK	
Water Strike	[Depth	Туре	Results/Samp	le Ref	. Depth (m)	Level (m)	Legend	2	Stratur	n Descri	ption		
	1.4	0 - 1.70	В	Sample B	33	1.10 1 3.10 1 3.70 1	193.95		Spongy brov	andy gravelly S ent. Cobbles a Material satur	sandy S ounded i illt/Clay i nd bould ated upo	with high cob ders are sub- on excavation	low cobble	2
Remark	<s: .<="" td=""><td>Groundwate Trial pit terr</td><td>er - stea ninated</td><td>dy flow of gro due to instabi</td><td>undwat lity of e:</td><td>er at 2.8m bo xcavation fac</td><td>jl es</td><td>1</td><td></td><td></td><td></td><td></td><td>Plant Use 12tN Track</td><td>d: ed</td></s:>	Groundwate Trial pit terr	er - stea ninated	dy flow of gro due to instabi	undwat lity of e:	er at 2.8m bo xcavation fac	jl es	1					Plant Use 12tN Track	d: ed
													Excavato	r

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Project		Derrinlou	ugh W	ind Farm			Projec	t No.	Coords (E,N):	6061	93.00	714987.00	Date	
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		Samol	les & In S	Situ Testina					4.20				GK	
Water Strike		Depth	Туре	Results/Sam	ole Ref	Depth (m)	Level (m)	Legend		Stratu	m Descri	ption		
	3.	.30 - 3.50	B	Sample R	34	2.60 1 4.20 1	92.28		Spongy bro	y soft, grey very boulder conten d angular. Mate	v gravelly t. Cobble prial was	amorphous Pea	y with low are sub-	
Remark	S:	Groundwat	er - stea minated	ady flow of gro	undwat	er at 3.4m bg	I						Plant Use	d:
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Cilent.	I		NUTA					_	2.80			GK	
Water Strike		Samp Depth	les & In S Type	itu Testing Results/Samp	ole Ref	Depth (m)	Level (m)	Legend		Stratum Des	scription		
Remark	1.8	0 - 2.00 Groundwat	B er - stea	Sample F	35	0.60 0.80 2.10 2.80	192.74 192.54 191.24 190.54		Spongy black/b Soft light brown Soft and firm lig cobble and bou rounded and ar boulder conteni and angular. Ma	rrown amorphous	Peat Clay avelly Silt/Clay w bbles and boulder ay with high cobb ulders are sub-ro ted upon excave 2.800m	ith low rs are sub- le and ounded tion.	2
Remark	KS:	Groundwat Trial pit teri	er - stea minated	dy flow of gro due to instabi	undwat lity of e:	er at 2.2m bę xcavation fac	gi xes					Plant Used:	: d
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						0.20	202.05 201.75	$\frac{1}{2} \frac{1}{2} \frac{1}$	Firm amorpho	ius Peat vn/grey sandy Sil light grey sandy <u>c</u>	lt/Clay gravelly Silt/Clay w	ith low	
	2.70 199.55								cobble conten boulders are s material was s	ight girly sample	auder content. Cob angular. At base o xcavation	f trial pit,	2
						2.70	199.55			End of Pit a	at 2.700m		3
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ke fe		Sampl	es & In S	itu Testing		Depth	Level	Lanard		Obertury Deser	· - • ·		
Stri	[Depth	Туре	Results/Sample	Ref	(m)	(m)	Legend		Stratum Descr	ipuon		
	2.1	0 - 2.30	В	Sample B6		0.40 1 0.60 1 2.40 1	89.86 89.66 187.86		Spongy brown a Firm light brown Firm, locally soft cobble and bould rounded.	morphous Peat sandy Silt/Clay , light grey sandy g der content. Cobbl der content. Cobbl gular End of Pit at 3.	gravelly Silt/Cl es and boulde gravelly Silt/Cl e and boulder: 600m	ay with low rs are sub-	
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	3.	Depth 20 - 3.40	В	Results/Sam	ble Ref	(m) 0.80 f 1.00 f 3.40 f	(m) 180.23 180.03		Spongy brown at Firm, locally soft gravelly Silt/Clay Cobbles and bou Firm, locally stiff, Clay with high cc boulders and sut material was sat	and stiff, light brow with medium cobi ilders and sub-rou light grey very sa bble and boulder p-rounded and angurated upon excav	wn very sandy y ole and boulden nded and angu ndy very grave content. Cobble jular. At base o ation	very content. llar. Ily Silt/ se and f trial pit,	2
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						1.10	164.00		Spongy brown	wn amorphous amorphous Pea	Peat at	300m			
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								4.10			GK	
Water Strike	Depth	Type	Results/Samp	le Ref	Depth (m)	Level (m)	Legend		Stratum Desci	ription		•
	3.80 - 4.00	В	Sample B	8	1.50 1 3.30 1 4.00 1 4.10 1	175.40 173.60 172.90 172.80		Spongy brown/l	orphous Peat	eat		
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	3.70 - 3.90	В	Sample B9	1.40	183.36 181.16 180.56	whe shte sh	Spongy bro Spongy an Spongy an Soft, locally low cobble sub-rounde saturated u	y firm, light grey and boulder con ed and angular. <i>i</i> upon excavation	sandy ve ntent. Col At base o	Peat Pry gravelly Silt/ bble and bould f trial pit, mater	/Clay with ers are rial was	
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50		Sampl	les & In S	Situ Testing				3.30			GK	
Wate Strik		Depth	Туре	Results/Sample Ref	Depth (m)	(m)	Legend		Stratum Descri	iption		
	2	.00 - 2.20	В	Sample B11	0.60 0.70 1.20 3.30	160.48 160.38 159.88		Spongy brown ar Soft to firm light t Firm grey/brown and boulder cont rounded and ang Firm and stiff sar and boulder cont rounded and ang	prown/white Silt/Cl slightly gravelly Si ent. Cobbles and I ular idy very gravelly S ent. Cobbles and I ular End of Pit at 3.3	ay lit/Clay with low boulders are su silt/Clay with hig boulders are su	cobble b-	1
Remark	s:	Groundwat Trial pit terr	er - slov ninated	v seepage of ground due to presence of	water at 3.1m boulders	bgl					Plant Used:	
											12tN Tracked Excavator	d

	Γ		Fehily	/ Timoney & Co. le Grainstore	Tel: +3	53-59-9723800							TrialPit	No
	EE		Sin Ba	igletons Lane agenalstown Co. Carlow	Email www.fe	l: info@ftco.ie Web: ehilytimoney.ie			Trial I	Pit Lo	Ŋ		TP1	5
	гім	ONEY	R21	XA66 Ireland									Sheet 1	of 1
Project Name:		Derrinlou	ugh Wi	ind Farm			Projec	t No.	Coords (E,N):	6090	92.00	714984.00	Date	
							1901		Level:			161.54	Scale	J19
Locatio	n:	County O	offaly						(m):				1:25	
Client:		Bord na N	Nóna						Depth 3.70				Logge GK	d
ke r		Sampl	les & In S	Situ Testing		Depth	Level			Otratu				
Wa Stri		Depth	Туре	Results/Samp	ole Ref	(m)	(m)	Legend		Stratur	m Descri	ption		1
						1.70 2.60 2.90	159.84 158.94 158.64	whe shie shie shi is shie	Spongy brov Soft light brov Firm, locally	wn amorphous own sandy Silt/ r soft, light grey	Peat Clay	Silt/Clay		2
						3.70	157.84	× × × × × × × × × × × × × × × × × × ×		End of	Pit at 3.7	'00m		4
Remark	ks:	Groundwat Trial pit teri	er - stea minated	dy flow of gro due to the dep	undwat oth of d	er at 3.3m bç ig	gl	1	1				Plant User 12tN Track Excavato	d: ed r

National Materials Testing Laboratory Ltd.

							•••							
				Particle			Index Pro	perties			Maximum	Minimum	Lab	
BH/TP	Depth	sample	Moisture	Density	<425um	LL	PL	PI			Density	Density	Vane	Remarks
No	m	No.	%	Mg/m3	%	%	%	%			Mg/m3	Mg/m3	kPa	
TP1	2.1-2.2	B1	9.2											Insufficient for LL &PI
TP4	3.3-3.5	B4	17.9		50.3	25	16	9						
TP5	1.8-2.0	B5	18.6		59.5	24	17	7						
TP7	2.1-2.3	B6	12.3		48.1	20	13	7						
TP8	3.2-3.40	B7									2.14	1.55		
TP10	3.8-4.0	B8	54.1		99.3	55	26	29						
TP12	3.7-3.9	B9	15.4		26.6	25	18	7						
TP13	3.8-4.0	B10	13.8		38.4	22	15	7						
TP14	21-2.3	B11	8.2		55.0	18	11	7						
											ļ			
NMTL		Notes :									Job ref No.	NMTL2856		
			1. All BS te	ests carried	d out using p	oreferred (definitive) r	nethod ur	less otherw	vise stated.	Location	Derrinloug	h Wind Farm. C	o. Offlay

SUMMARY OF TEST RESULTS



















Appendix F

Ground Investigation (August/October/November 2019): Trial Pit Logs and Photographs









		Fehily Timor The Grai Singleton Bagenal Co. Ca R21 XA66	ney & Co. instore Tel: +35 is Lane Email: Istown arlow www.fe 3 Ireland	3-59-9723800 info@ftco.ie Web: shilytimoney.ie		•	Trial Pi	t Log		TrialPit No BP1/TP ⁴ Sheet 1 of 1
Project	Derrinler	uarla \A/inad I	F orma		Projec	t No.	Coords (E,N):	608812.61	716183.12	Date
Name:	Demnio	ign wind i	rann		P1990)	Level:			12/08/2019
Location	n:						Dimensions			Scale
							Depth			Logged
Client:							3.00			IH
Water Strike	Samp Depth	les & In Situ Te Type Res	esting sults/Sample Ref	Depth (m)	Level (m)	Legend		Stratum Descri	ption	
				2.45		will will a solic	Soft brown fibrou Grey slightly clay	s PEAT, colour ch	with rare cobble	es 3
Remark	s: Pit dry, stal	ble		I						Plant Used:

	FEHI		Fehily Th Sin Ba (R21	r Timoney & Co. e Grainstore gletons Lane agenalstown Co. Carlow XA66 Ireland	Tel: +3 Email www.fe	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial F	Pit Lo	bg		TrialPit N BP1/T Sheet 1 o	№ P2
Project	C)errinloı	Jah Wi	nd Farm			Projec	t No.	Coords (E,N):	6088	64.62	716178.21	Date	
Name:			-g				P1990		Level:				12/08/20	19
Location	n:								(m):				1:25	
Client:									Depth				Logged	1
Water Strike		Sampl	les & In S	itu Testing	le Ref	Depth (m)	Level (m)	Legend		Stratun	n Descri	ption		
		epin	Type	Results/Samp		2.20			Grey slightly subrounded,	silty slightly gr limestone.	Pit at 2.2	AND. Gravel is		2
Remark	ks: F a	Rapid wate It 1.5m	r ingres:	s at 1.8m, side	e unstal	ble below 1.	8m TP e	excavated	in base of existir	ng borrow are	ea Sam	ple taken	Plant Used	:

	FEHILY	Fehily Tr Sir B R2	y Timoney & Co. he Grainstore ngletons Lane agenalstown Co. Carlow I XA66 Ireland	+353-59-9723800 ail: info@ftco.ie Web: v.fehilytimoney.ie			Trial P	it Log		TrialPit N BP3/T Sheet 1 (NO FP1
Project			· · -		Projec	t No.	Coords (E,N):	610917.03 71	4258.39	Date	
Name:	Derrinle	ough W	ind Farm		P1990		Level:			13/08/20	019
Locatio	m.						Dimensions			Scale	
Locatio							(m):			1:25	
Client:							0.20			Logged	d
Water Strike	San Depth	nples & In S	Situ Testing Results/Sample Ref	Depth f (m)	Level (m)	Legend		Stratum Description	1		
						alte alte al te alte alte	Soft brown PE	AT			
				0.20		, she , she , sh		End of Pit at 0.200m			
											-
											-
											-
											-
											' -
											-
											-
											-
											-
											2
											-
											-
											-
											-
											-
											-
											3 -
											-
											-
											4 -
											-
											-
											-
Remark	ks:					1	<u> </u>			Plant Liser	1
											••

	FEHILY	Fehily Th Sin Ba (R21	/ Timoney & Co. le Grainstore lgletons Lane agenalstown Co. Carlow 1 XA66 Ireland	Tel: +353 Email: i \ www.feh	3-59-9723800 info@ftco.ie Web: nilytimoney.ie		•	Trial I	Pit Lo	р		TrialPi BP3/ Sheet 1	t No TP2 I of 1
Project						Project	t No.	Coords (E,N):	6108	374.79	714262.9	07 Dat	e
Name:	Derrinlou	igh Wi	ind Farm			P1990		Level:				13/08/2	2019
Location	ŋ.					1		Dimensions				Sca	le
								(m):				1:2	5
Client:								3.00					ea
Water Strike	Sampl Depth	les & In S Type	itu Testing Results/Samp	le Ref	Depth (m)	Level (m)	Legend		Stratu	m Descri	iption		
Pennetic					0.40			Soft brown f	PEAT soft grey slight End of	ly sandy	SILT firm at	1.5m	
Remark	s: No sample	taken										Plant Use	ed:

		Fehil Ti Sir B R2	y Timoney & Co. ne Grainstore Tel: +3 ngletons Lane Emai agenalstown Co. Carlow www.f 1 XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie		•	Trial Pi	t Lo	9	TrialPit N BP5/T Sheet 1 of	^{lo} ' P1 ₁1
Project	Dorrinlo		ind Earm		Projec	t No.	Coords (E,N):	616827.	00 717147.00	Date	
Name:	Derrind	ugn w			P1990)	Level:			07/10/20	19
Locatio	in:						Dimensions]	Scale	
Client							Depth			Logged	
	0						1.80			IH	
Water Strike	Depth	Type	Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum D	escription		
				1.80			Grey clayey sand cobbles	End of Pit	at 1.800m	unded	2
Remar	ks: Samples	taken at	1m							Plant Used:	:
									L		

	Г		Fehily	Timoney & Co.	Tel: +3	53-59-9723800							TrialPit N	No
			Sin Ba	gletons Lane genalstown Co. Carlow	Email www.fe	l: info@ftco.ie Web: ehilytimoney.ie		•	Trial Pi	it Lc	bg		BP6/T	P1
· ·	ТІМ	ONEY	R21	XA66 Ireland		, , , , , , , , , , , , , , , , , , ,							Sheet 1 c	of 1
Project		Derrinlou	ıah Wi	nd Farm			Project	No.	Coords (E,N):	60648	89.82	714489.73	Date	
iname:			5				P1990		Level:				15/08/20)19
Locatio	n:								Dimensions (m):	Г			Scale	
Client									Depth				Logged	d
									1.00				IH	
Water Strike		Depth	Type	Results/Samp	ole Ref	Depth (m)	Level (m)	Legend		Stratum	Descrip	otion		
						1.00			Firm grey gravel limestone	End of P	Pit at 1.0	DOm	ders of	
Remark	ks:												Plant Used	 I:

		Fehily Th Sir B R2 ⁻	y Timoney & Co. he Grainstore gletons Lane agenalstown Co. Carlow I XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial Pit	Log		TrialPit No BP7/TP1 Sheet 1 of 1
Project	Derrinlo	uah W	ind Farm		Projec	ct No.	Coords (E,N):	610264.47	715941.17	Date
Name:		-g			P1990)	Level:			12/08/2019
Locatio	n:						Dimensions (m):			1:25
Client:							Depth			Logged
ъ е	Samp	oles & In S	Situ Testing	Depth			2.90			
Wat Strij	Depth	Туре	Results/Sample Ref	(m)	(m)	Legend	Soft brown PEAT	Stratum Descri	ption	
				0.20		k sta sta sta sta sta	0.64.5		-	
							Soft to firm grey sa subangular cobble	andy gravelly SIL s and boulders	I with subround	led to
						$\times \times \times \times$				
						(***** *****				
										1 -
						$\times \times \times \times$				
						(* * * * * * * * * *				
						$\langle \times \times \times \times \times \rangle$				
						$\times \times \times \times$				
										2 -
						(* * * * * * * * * *				
						$\times \times $				-
						$\overset{\circ}{\times}\overset{\times}{\times}$				
				2.90		<u>`******</u>		End of Pit at 2.9	900m	
										3
										-
										4 -
Remark	(S:	•		· 1						Plant Used:
		Fehily Th Sin Ba (R21	Timoney & Co. e Grainstore Tel: + gletons Lane Ema igenalstown Co. Carlow www XA66 Ireland	353-59-9723800 ail: info@ftco.ie Web: .fehilytimoney.ie		•	Trial Pi	t Log		TrialPit No BP8/TP1 Sheet 1 of 1
--------------	---------------	---------------------------------------	---	--	--------------	----------	--	----------------	-------------	----------------------------------
Project	Derrinlo	uah Wi	nd Farm		Projec	ct No.	Coords (E,N):	614794.00	719548.00	Date
Name:					P1990)	Level:			08/10/2019
Locatio	n:						Dimensions (m):			Scale 1:25
Client:							Depth			Logged
er	Samp	les & In S	itu Testing	Donth			3.00			
Water Strike	Samp Depth	Iles & In S	itu Testing Results/Sample Ref	Depth (m) 1.10 2.50 3.00	Level (m)	Legend	Soft brown fibrou Soft grey slightly @2m, becoming Firm grey and bu	Stratum Descri	ption AY	
										4
Remark	ks: Groundwa	l ter seepa	age at 3m			<u> </u>				Plant Used:

	Г	Fehily	/ Timoney & Co.	Tel: +3	53-59-9723800						TrialPit N	٩o
		Sir	agenalstown	Email	l: info@ftco.ie Web:		•	Trial P	it Log		BP8/T	P2
	TIMONEY	R21	XA66 Ireland		eninytimoney.ie						Sheet 1 o	of 1
Project	Derrinlo	ouah W	ind Farm			Project	No.	Coords (E,N):	614815.00	719370.00	Date	
Name:						P1990		Level:			08/10/20	19
Locatio	n:							Dimensions (m):			Scale	
Client:								Depth			Loggec	ł
	0							2.60			<u> </u>	
Water Strike	Depth	Type	Results/Sam	ple Ref	. Depth (m)	Level (m)	Legend		Stratum Desc	cription		1
					0.10 2.20 2.60			Brown peaty TC Soft to firm grey subangular to su Firm grey lamina	PSOIL slightly sandy CL ubrounded gravel ated CLAY End of Pit at 2	AY with occasion	nal	
Remark	<s:< td=""><td><u> </u></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>Plant Used</td><td>:</td></s:<>	<u> </u>									Plant Used	:

	FEHILY	Fehil Ti Sii B R2	y Timoney & Co. he Grainstore ngletons Lane lagenalstown Co. Carlow 1 XA66 Ireland	Tel: +3 Email www.fe	53-59-9723800 : info@ftco.ie Web: ehilytimoney.ie		•	Trial P	it Log		TrialPit No BP9/TP1 Sheet 1 of 1
Project			nu d E			Project	t No.	Coords (E,N):	206008.00	215159.00	Date
Name:	Derrin	ough W	ind Farm			P1990		Level:			06/11/2019
Locatio	in:							Dimensions			Scale
								(m):			1:25
Client:								2.10			Logged
Water Strike	Sa Depth	Type	Situ Testing Results/Samp	ole Ref	Depth (m)	Level (m)	Legend	Firm brown/blac	Stratum Descr ck amorphous Peat	iption	
					0.70			Firm light browr	1/orange clayey Silt		1
					2.10				End of Pit at 2.	100m	3 -
Remark	ks: No grou excavate	ndwater er e further. T	ncountered. Tri rial pit noted a	al pit te s stable	rminated du	e to pre	sence of t	ooulders at base of	trial pit and unat	ble to	Plant Used:

		Fehily Th Sin Ba (R21	r Timoney & Co. e Grainstore gletons Lane agenalstown Co. Carlow XA66 Ireland	Tel: +35 Email www.fe	53-59-9723800 : info@ftco.ie Web: ehilytimoney.ie			Trial P	it Log		TrialPit N BP9/T Sheet 1 c	No P2 of 1
Project	Derrinlou	ah Wi	ind Earm			Projec	ct No.	Coords (E,N):	206008.00	215159.00	Date	-
Name:	Derrinou		nu Fann			P1990)	Level:			06/11/20)19
Location	n:							Dimensions (m):			Scale	
Client [.]								Depth			Logged	d
	Sample	es & In S	Situ Testina					3.00				
Water Strike	Depth	Туре	Results/Samp	ole Ref	Depth (m)	Level (m)	Legend		Stratum Descri	ption		•
					1.50 1.80 3.00		shie shie	Firm brown amo	orphous Peat	with occasional th.	cobbles.	
Remark	s: No groundv	vater en	countered.								Plant Used	4

		Fehily Th Sir B R2 ⁻	y Timoney & Co. le Grainstore Igletons Lane agenalstown Co. Carlow XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial Pit Log	9	TrialPit No SS/TP1 Sheet 1 of 1
Project Name:	Derrinlo	ugh W	ind Farm		Projec	ct No.	Coords (E,N): 611002.0	07 718600.21	Date
					F 1990)	Dimensions		Scale
Locatio	n:						(m):		1:25
Client:							Depth 2.20		Logged IH
Water Strike	Samı Depth	oles & In S	Situ Testing Results/Sample Ref	Depth (m)	Level (m)	Legend	Stratum De	escription	
				0.30			Very soft dark brown PEAT Firm grey slightly sandy CLAY to subrounded cobbles End of Pit a	with occasional sub	angular 1 - 2 - 3 - 4 -
Remark	ks: Moderate	l groundw	ater inflow at 2.2m	1					Plant Used:

	FEH		Fehily Th Sin Ba (R21	Timoney & Co. e Grainstore gletons Lane igenalstown Co. Carlow XA66 Ireland	Tel: +35 Email www.fe	53-59-9723800 : info@ftco.ie Web: ehilytimoney.ie			Trial P	it Log		TrialPit No SS/TP2 Sheet 1 of 1
Project		Derrinlou	gh Wi	nd Farm			Projec	ct No.	Coords (E,N):	610958.79	718511.74	Date
							P1990)	Level:			16/08/2019 Scale
Locatio	on:								(m):			1:25
Client:									Depth 3.00			Logged IH
Water Strike	[Sample Depth	es & In S Type	itu Testing Results/Samp	le Ref	Depth (m)	Level (m)	Legend		Stratum Descri	iption	
						1.10		shile, shile, shi shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile, shile	Very soft dark to subangular to gravelly 2.1m -	Slightly sandy CLAY subrounded cobbles occasional boulders End of Pit at 3.0	with occasional 1.9m firm and s s	1
Remark	ks:	Slow to mo	derate g	roundwater in	flow at	2.3m						Plant Used:

		Fehily Th Sir Ba (R21	/ Timoney & Co. le Grainstore Tei Igletons Lane E agenalstown Co. Carlow w I XA66 Ireland	l: +353-59-9723800 Email: info@ftco.ie Web: ww.fehilytimoney.ie			Trial P	it Log		TrialPit No SS/TF Sheet 1 of	⊳ >3 1
Project	Derrinlo	iah W	ind Farm		Proje	ct No.	Coords (E,N):	610923.85	718598.36	Date	
Name:	Derrinot				P199	0	Level:			16/08/201	19
Location	n:						Dimensions (m):			Scale 1:25	
Client:							Depth			Logged	
ы e	Sampl	es & In S	Situ Testing	Danth	Laval		3.20			IH	
Wate Strik	Depth	Туре	Results/Sample R	tef (m)	(m)	Legend		Stratum Descri	ption		
				1.10 1.30 1.50 3.20			Very soft dark b Very soft grey sl Very soft brown Soft grey sandy 2m - firm, freque	ightly sandy CLAY PEAT CLAY with occasio ent cobbles 2.4m - o	nal subrounde occasional bou	d cobbles Iders	2
Remark	(s: Pit dry eide	as etable	<u> </u>						1	Direction	
Remark	s: Pit dry, side	es stable	9							Plant Used:	

		Fehily The Sing Bay C R21	Timoney & Co. e Grainstore Tel: + Jletons Lane Emi genalstown o. Carlow www XA66 Ireland	353-59-9723800 ail: info@ftco.ie Web: v.fehilytimoney.ie			Trial P	it Lc	g		TrialPit No SS/TP4 Sheet 1 of 1
Project	Derrinlo	iah Wi	nd Farm		Projec	t No.	Coords (E,N):	61093	9.87	718751.21	Date
Name:	Berninoe	ign m			P1990)	Level:				16/08/2019
Locatio	n:						Dimensions (m) [.]	Г			Scale
Client							Depth				Logged
Client:						-	2.70				IH
Water Strike	Sampl Depth	es & In Si Type	Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum	Descri	ption	
				0.15			Soft dark brow Firm grey sligh subrounded co	n fibrous PEA tty sandy gra bibbles 2m - fir End of F	velly CL m to sti	AY with occasi ff	onal 1 · 2 · 3 ·
Remark	<s: clay="" damp<="" td=""><td>to 2m bg</td><td>)I</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>Plant Used:</td></s:>	to 2m bg)I								Plant Used:

	FEHILY	Fehily T The Singl Bag Co R21 X	Timoney & Co. Grainstore T etons Lane enalstown). Carlow (A66 Ireland	īel: +353-59-9723800 Email: info@ftco.ie Web: www.fehilytimoney.ie			Trial P	it Lo	bg		TrialPit No TP/CC Sheet 1 of 1	1
Project	Derrinler				Projec	ct No.	Coords (E,N):	6062	61.61	714501.70	Date	
Name:	Derrinio	ugn win	ia Farm		P1990)	Level:				15/08/2019	
Locatio	n:						Dimensions				Scale	
Client							 Depth				Logged	
Client:							3.40				IH	
Vater Strike	Samp	ies & in Siti		Depth (m)	Level (m)	Legend		Stratu	m Descri	iption		
	Depth	Type	Results/Sample	Ref (III) 3.30 3.40		silve silve silve solve silve silve silve solve	Very soft dark t	T with large End of	boulder Pit at 3.4	EAT	1	
Remark	s: Pit dry, sta	ble				<u> </u>	<u> </u>				Plant Used:	

		Fehily Th Sin Ba (R21	/ Timoney & Co. le Grainstore Tel: +3 igletons Lane Ema agenalstown Co. Carlow www: I XA66 Ireland	353-59-9723800 il: info@ftco.ie Web: fehilytimoney.ie			Trial Pit	Log		TrialPit No TP/CC Sheet 1 of 1	2
Project	Derrinlo	uah Wi	ind Farm		Projec	ct No.	Coords (E,N):	607634.07	715108.69	Date	
Name:	Dernino	ugii w			P1990	ט	Level:			14/08/2019	ł
Locatio	n:						Dimensions	[]	Scale	
Client							Depth			Logged	
Client:							2.30			IH	
Water Strike	Samı Depth	oles & In S	Situ Testing Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum Descrip	otion		
				2.30		shle shle sh e shle shle shle shle shle shle shle shle shle shle shle	Very soft dark brown	T/CLAY with occ and boulders 2	casional suban .3m - boulders	gular to more	2
Remark	ks: Slow wate	r inflow a	at 2.3m	· ·						Plant Used:	

			IN	e Grainslore		1 EU UZUUUU							
			Sin	gletons Lane genalstown	Email:	info@ftco.ie Web:		-	Trial Pi	t Loa		TP/C	C3
	FE TIM	HILY Oney	(R21	Co. Carlow XA66 Ireland	www.fe	hilytimoney.ie						Sheet 1 c	of 1
Projec	t	Derrinlou	iah Wi	nd Farm			Projec	t No.	Coords (E,N):	609190.66	715551.75	Date	
Name:		Bonnioe	-9				P1990		Level:			12/08/20)19
Locatio	on:								Dimensions (m):			Scale 1:25	
Client:									Depth			Logged	d
л e		l Sampl	es & In S	itu Testing		Dopth	Loval		3.50				
Water		Depth	es & In S	itu Testing Results/Samp	le Ref	Depth (m)	Level (m)	Legend	Soft brown PEAT	Stratum Descr	vel below 2.5m	: more rounded	2
						3.00				End of Pit at 3.	500m		4
Remai		Slow seepa	age at 3.	5m Sample ta	ken at 2	2.0m		<u> </u>				Plant Used	l ::

		Fehil Ti Si B R2	y Timoney & Co. he Grainstore Te nagletons Lane I agenalstown Co. Carlow w 1 XA66 Ireland	el: +353-59-9723800 Email: info@ftco.ie Web: ww.fehilytimoney.ie			Trial Pit Log	TrialPit No TP/CC4 Sheet 1 of 1
Project	Derrin		ind Farm		Projec	t No.	Coords (E,N): 610220.87 715671.63	Date
Name:	Denni				P1990		Level:	14/08/2019
Locatio	in:						Dimensions	Scale
Client							Depth	Logged
							2.30	IH
Water Strike	Depth	Type	Situ Testing Results/Sample F	Cef Depth (m)	Level (m)	Legend	Stratum Description	
				2.30			Soft dark brown PEAT Soft grey sandy SILT with occasional subangular subrounded cobbles	
Remar	ks: Pit dry							Plant Used:

		Fehily Th Sir Ba C R21	/ Timoney & Co. le Grainstore Tei lgletons Lane E agenalstown Co. Carlow w XA66 Ireland	: +353-59-9723800 imail: info@ftco.ie Web: ww.fehilytimoney.ie			Trial Pi	it Log		TrialPit No TP/ CC5SS Sheet 1 of 1	5
Project	Deminte				Projec	t No.	Coords (E,N):	609071.50	715374.15	Date	
Name:	Derrinio	ugn w	ind Farm		P1990)	Level:			12/08/2019	
Locatio	n:						Dimensions			Scale	
							(m): Depth			1:25	
Client:							2.10			IH	
Water Strike	Sam Depth	ples & In S	Situ Testing Results/Sample R	Depth ef (m)	Level (m)	Legend		Stratum Descri	ption		
				2.10		shia shia hi shia shi shia shia shia shia shia shia shi shia shia shi shia shia shi shia shia shia shia shia shia shia shia shia shia shi shia shi shi shi shi shia shi shia shi shia shia shi shia shi shia shi shia	Soft dark brown Firm grey slightly boulders. Boulde Limestone.	to brown PEAT y sandy gravelly SII ers are subangular End of Pit at 2.1	_T with occasior to subrounded	nal 1	2
Remark	ks: Pit dry, sta	 able Hard	digging at 2.1m							Plant Used:	

	FEHILY	Fehily Th Sir Bi R21	/ Timoney & Co. ne Grainstore ngletons Lane agenalstown Co. Carlow I XA66 Ireland	Tel: +3 Email www.fe	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie		•	Trial P	it Lo	bg		TrialPit No TP/CSH Sheet 1 of 1	-11
Project	Deminter					Projec	ct No.	Coords (E,N):	6078	55.94	715110.28	Date	
Name:	Derrinio	ugn w	Ind Farm			P1990)	Level:				14/08/2019	,
Locatior	n:							Dimensions				Scale	
								_(m): 				1:25	
Client:								2.85				IH	
Water Strike	Samp Depth	les & In S Type	Situ Testing Results/Sam	ole Ref	Depth (m)	Level (m)	Legend		Stratun	n Descri	ption		
Pemark					2.85			Very soft dark t	End of	Pit at 2.8	AT		1
Remark	s: Pit stopped	due to	possible arch	eology a	at 2.85m							Plant Used:	

	Г		Fehily	Timoney & Co. e Grainstore	Tel: +3	53-59-9723800		-					TrialPit I	No
		1	Sin Ba	gletons Lane genalstown	Email	: info@ftco.ie Web:		-	Trial F	Pit Lo	Ŋ		TP/CS	SH2
		EY	R21	XA66 Ireland					1				Sheet 1 o	of 1
Project	De	errinlou	ıgh Wi	nd Farm			Project	No.	Coords (E,N):	6080	13.05	714987.63	Date	
indiric.							P1990		Level:				14/08/20	019
Locatio	n:								(m):				1:25	
Client:									Depth				Logge	d
ы e		Sampl	es & In S	itu Testing		Denth			5.50					
Wat Strik	Dep	oth	Туре	Results/Sampl	le Ref	(m)	(m)		Light brown o	Stratur	n Descri	ption		1
									Light brown g					
						0.30								-
									Firm grey bro boulders	wn gravelly C	lay with	frequent suban	igular	-
														-
								- <u>-</u> -0						-
														-
														-
								6-0-1						1 -
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														2
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								<u> </u>						-
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								<u> </u>						
														-
														-
								 						3 -
														-
														-
														-
						3.50		<u> </u>		End of	Pit at 3.5	500m		-
														-
														-
														-
														4 -
														-
														-
Remark	ks:												Plant Used	1:

-		Fehily Th Sir Ba (R21	r Timoney & Co. le Grainstore Tel: +: lgletons Lane Ema agenalstown Co. Carlow www. XA66 Ireland	353-59-9723800 iil: info@ftco.ie Web: fehilytimoney.ie			Trial P	it Log		TrialPit N TP/T Sheet 1 o	lo 1 f 1
Project	Derrinlo	uah W	ind Farm		Projec	t No.	Coords (E,N):	607026.50	715375.32	Date	
Name:	Derrinio				P1990)	Level:			16/08/20	19
Locatio	n:						Dimensions (m):			Scale 1:25	
Client:							Depth			Logged	1
е е	Samp	les & In S	Situ Testing	Denth			3.00				
Wat Stril	Depth	Туре	Results/Sample Ref	(m)	(m)	Legend		Stratum Descri	ption		
				1.10		alike alike alike alike alike alike alike alike alike alike alike alike alike alike	Very soft brown	PEAT ery sandy CLAY End of Pit at 3.0	000m		2
Remark	s: Slow grour	ndwater	inflow at 2.3mbgl	<u> </u>		1				Plant Used	:
			on at 2.0mbgr								

		Fehily Tr Sir Bi R21	/ Timoney & Co. he Grainstore ngletons Lane agenalstown Co. Carlow I XA66 Ireland	Tel: +353-59-972 Email: info@ftca Web: www.fehilytimone	3800 p.ie ey.ie			Trial P	it Lo	bg		TrialPi TP/ Sheet 1	t No T2 of 1
Project	Derrinlo	uah W	ind Farm			Projec	t No.	Coords (E,N):	6072	39.98	714755.5	6 Dat	е
Name:		ugii II				P1990)	Level:				16/08/2	2019
Locatio	on:							Dimensions (m):		[Sca 1:2	e 5
Client:								Depth				Logg	ed
50	Sam	oles & In S	Situ Testina					2.30				' IH	
Wate	Depth	Туре	Results/Sample	Ref (m)		Level (m)	Legend		Stratun	n Descri	ption		
				1.20			shite shite shite e shite shite shite shite shite shite shite shite shite shite shite e shite shite shite shite shite shite shite shite shite shite shite shite shite sh	Firm grey slightly sandy slightly gravelly CLAY w occasional subangular to subrounded cobbles, s 2mbgl End of Pit at 2.300m				vith stiff at	1
													3
Remar	ks: Hard dig, s	sample ta	aken at 1.8m									Plant Use	ed:
											L		

Т		Fehily Th Sin Ba (R21	r Timoney & Co. e Grainstore Tel: +3 gletons Lane Emai agenalstown Co. Carlow www.f XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie		•	Trial Pit Log		TrialPit No TP/T3 Sheet 1 of 1
Project	Dorrinlo		ind Form		Project	No.	Coords (E,N): 6066666.00	713348.00	Date
Name:	Deminio	ugn w	nu Farm		P1990		Level:		07/10/2019
Location	:						Dimensions]	Scale
							Depth		Logged
Client:							2.80		IH
Water Strike	Samp Depth	les & In S Type	Situ Testing Results/Sample Ref	Depth (m)	Level (m)	Legend	Stratum Desc	ription	
				0.50			Soft to firm brown fibrous PEAT Soft grey very sandy SILT with oc subrounded cobbles	casional subangu	Ilar to
Remarks	s: Sides of ex	cavatior	n unstable	2.80			End of Pit at 2	.800m	3 3 4 Plant Used:

		Fehily Th Sir Bi R21	/ Timoney & Co. le Grainstore Tel Igletons Lane E agenalstown Co. Carlow ww I XA66 Ireland	: +353-59-9723800 mail: info@ftco.ie Web: ww.fehilytimoney.ie			Trial Pit	Log		TrialPit M TP/T Sheet 1 c	1 0 4 0f 1
Proiect	Deminle				Projec	t No.	Coords (E,N):	605956.56	713099.06	Date	
Name:	Derrinio	ugn w	Ind Farm		P1990)	Level:			16/08/20	19
Locatio	n:						Dimensions]	Scale	
							Depth			Logged	ł
Client:							3.00			IH	
Water Strike	Samp Depth	Type	Situ Testing Results/Sample R	Depth ef (m)	Level (m)	Legend		Stratum Descri	ption		
Remark	Ks: Sample ta		5m	1.40 2.20 3.00		alle alle alle alle alle alle alle alle	Very soft brown PE	slightly organic to a	SILT (MARL)	r to	
Remark	ks: Sample tal	ken at 2.	5m							Plant Used	:

		Fehily Tr Sir Bi R21	y Timoney & Co. ne Grainstore Tri agletons Lane agenalstown Co. Carlow v 1 XA66 Ireland	el: +353-59-972380(Email: info@ftco.ie Web: www.fehilytimoney.ie)		Trial P	it Log)	TrialPit M TP/T Sheet 1 c	No 5 Dof 1
Project	Derrinlo		ind Farm		Projec	ct No.	Coords (E,N):	606961.0	0 712823.00) Date	
Name:	Derrino				P1990)	Level:			07/10/20)19
Locatio	n:						Dimensions (m):			Scale	
Client:							Depth			Logged	d
	Somr		Situ Tosting				2.40			IH	
Water	Depth	Dies & In S	Situ Testing Results/Sample I	Ref Depth (m) 1.70	Level (m)	Legend site site site site site site	Soft brown fibro	Stratum Des bus PEAT	scription		
Remark	ks: Slow grou	ndwater	inflow at 2.4m							Plant Used	- - - - -
									L		

		Fehily Tr Sir Bi R21	/ Timoney & Co. le Grainstore lgletons Lane agenalstown Co. Carlow I XA66 Ireland	Tel: +35 Email www.fe	53-59-9723800 : info@ftco.ie Web: ehilytimoney.ie		·	Trial Pi	t Lo	bg		T s	FrialPit N P/T heet 1 o	lo '6 f 1
Project	Derrinlo	uah W	ind Farm			Projec	ct No.	Coords (E,N):	6064	52.00	714872.0	00	Date	
Name:		agn n				P1990)	Level:		_		0	7/10/20	19
Locatio	n:							Dimensions (m):				٦ L	Scale	
Client:								Depth					Logged	1
	Comr		Nity Testing					2.60					IH	
Water Strike	Depth	Туре	Results/Samp	le Ref	Depth (m)	Level (m)	Legend		Stratur	n Descri	ption			
					1.60		Alle, shile, shi	Soft brown fibroi	sandy gra Iders	Velly CL	AY with sub	rounded		2
Remark	ks: Slow grou	ndwater	inflow at 2.6m				·					Pla	nt Used:	
												L		

		Fehily Th Sir Ba (R21	/ Timoney & Co. he Grainstore ngletons Lane agenalstown Co. Carlow I XA66 Ireland	Tel: +353 Email: ir V www.fehi	-59-9723800 nfo@ftco.ie Veb: ilytimoney.ie			Trial Pi	t Log		TrialPit I TP/1 Sheet 1 d	No [7 of 1
Project	Derrinlo		ind Earm			Projec	t No.	Coords (E,N):	605698.00	714983.37	Date	
Name:	Derrinio	ugn w				P1990)	Level:			15/08/20	019
Location	n:							Dimensions (m):			Scale	
Client:								Depth			Logge	d
	Samr	les & In S	Situ Testina					2.90		,	IH	
Wate Strike	Depth	Туре	Results/Sampl	le Ref	Depth (m)	Level (m)	Legend	Very soft dark bro	Stratum Descri	ption		1
					1.20		$\begin{array}{c} \text{solve} so$	Very soft dark bro	sandy SILT with o	ccasional grav	el and	
Remark	s: Moderate	groundw	ater inflow at 2	.7m, san	nple taken	at 2m,	sides unst	able below 2.7m			Plant Usec	1:

		Fehily Timoney & The Grainstore Singletons Lan Bagenalstown Co. Carlow R21 XA66 Irelar	Co. e Tel: +3 e Emai www.f	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial Pi	t Log	g	TrialPit N TP/T Sheet 1 o	lo '8 f 1
Project	Dorrinlo	uch Wind For	~		Projec	t No.	Coords (E,N):	605917	.39 715616.94	Date	
Name:	Derrino				P1990		Level:			15/08/20	19
Locatio	n:						Dimensions			Scale	
Client							Depth			Logged	
							2.20			IH	
Client:	Depth	Dies & In Situ Testing Type Results/S	Sample Ref	Depth (m) 0.40	Level (m)		2.20 Soft brown PEAT Soft grey sandy (subrounded cobb	Stratum D	Description	r to firm to stiff	2
Remark	ks: Sides uns	table below 0.7m								Plant Used:	4

		Fehily Timor The Grai Singleton Bagenal Co. Ca R21 XA66	ney & Co. instore Tel: +3 is Lane Emai istown arlow www.f 5 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie		•	Trial Pi	t Log	J	TrialPit No TP/T9 Sheet 1 of 1
Project	Derrinlo	ugh Wind I	Farm		Projec	et No.	Coords (E,N):	605954.0	0 714337.00	Date
Name:		.g			P1990)	Level:			07/10/2019
Location	n:						Dimensions (m):			1:25
Client:							Depth			Logged
ь e	Sampl	es & In Situ Te	esting	Death	Laval		2.90			
Vate Strik	Depth	Type Res	sults/Sample Ref	(m)	(m)	Legend		Stratum Des	scription	
				1.90 2.10 2.90		Alte shite s	Soft brown fibrou Soft light grey sa Very clayey sand subrounded cobl	Indy CLAY Indy CLAY Indy GRAVEL with Des End of Pit at	occasional suban	gular to 3
Remark	s: Slow groun	dwater inflow	v at 2.9m	<u> </u>		<u> </u>				Plant Used:

	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Set Derrinlough Wind Farm			53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie		•	Trial P	it Log)	TrialPit No TP/T1 Sheet 1 of	D 0 1	
Project	Derrink		ind Farm			Projec	t No.	Coords (E,N):	605092.6	5 714561.4	2 Date	
Name:	Derrink	ugii u				P1990		Level:			15/08/201	19
Locatio	n:							Dimensions (m):			Scale 1:25	
Client:								Depth			Logged	
Le e	Sam	ples & In S	Situ Testing		Denth			3.20				
Water	Depth	ples & In S	Situ Testing Results/Sam	ble Ref	Depth (m)	Level (m)	Legend	Very soft dark t	Stratum De prown and brown	scription PEAT gravelly SILT w unded cobbles a	ith and	1 2 3 4
Remar	ks: Slow grou	Indwater	inflow at 3.2m	1							Plant Used:	-

	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-59-972 Email: info@ftco Web: www.fehilytimone ct Ct Ct Derrinlough Wind Farm						Trial P	it Log		TrialPit No TP/T11 Sheet 1 of 1
Project	Derrinlo	ugh W	ind Farm		Projec	ct No.	Coords (E,N):	605273.00	714022.00	Date
iname.		•			P1990	0	Level:			07/10/2019 Scale
Locatio	n:						(m):			1:25
Client:							Depth 2 20			Logged IH
ter ke	Sam	oles & In S	Situ Testing	Depth	Level	Langed		Otratura Daga	· 4'	
Wate	Image: Slow groundwater inflow at 2.1m					Legend	Soft to firm brow	Stratum Desci	y gravelly SILT, o	damp, 1 2 3 4
Remark	Slow grou	 ndwater	inflow at 2.1m							Plant Used:

т	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown CO. Carlow R21 XA66 Ireland Tel: +353-59-972 Email: info@ftc Web: www.fehilytimor				Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-59-9723800 Email: info@ftco.ie Web: www.fehilytimoney.ie					TrialPit No TP/T12 Sheet 1 of 1)
Project	Derrinlo	ugh W	ind Farm		Projec	ct No.	Coords (E,N):	609662.00	715257.00	Date	
iname.		•			P1990	0	Level:			08/10/2019 Scale	
Location	:						(m):			1:25	
Client:							Depth 2 20			Logged IH	
ke r	Samp	les & In S	Situ Testing	Depth	Level						-
Water Strike	Samples & In Situ Testing Depth Type Results/Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref Image: Sample Ref		Depth (m) 0.20 1.00 2.20	Level (m)	Legend	Soft dark brown Grey silty SANI Grey silty SANI	Stratum Descrip n PEAT D with frequent subro D with occasional sub End of Pit at 2.20	prounded cobbl	s 1		
Remarks	s: Rapid grou	Indwater	inflow around bould	ers in upper	<u>1m</u>					Plant Used:	

г	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-59-97236 Email: info@ftco.i Web: www.fehilytimoney				9-9723800 o@ttco.ie b: timoney.ie			Trial Pi	t Log		TrialPit N TP/T Sheet 1 o	10 13 11
Project	Derrinlou	ugh W	ind Farm		F	Projec	t No.	Coords (E,N):	609599.89 714502	2.47	Date	
iname.		•			F	P1990		Level:			13/08/20 Scale	19
Location	1:							(m):			1:25	
Client:								Depth 2.60			Logged IH	ł
ater ike	Samp	les & In S	Situ Testing	Depth	Le	evel	Legend		Stratum Description			
Wat	Depth	Туре	Results/Sample	2.60				Soft brown PEAT	Stratum Description I slightly sandy CLAY with oc ow 2m: some subangular to End of Pit at 2.600m	casiona	al gravel nded	2
Remark	s: Minor grou	ndwater	r inflow at 2m sa	imple taken at	1.5m						Plant Used	:

	Г	Fehil	y Timoney & Co.	1. +353 50 0702000							TrialPit N	lo
	FEHILY	Sir B	agenalstown Co. Carlow w	Email: info@ftco.ie Web: www.fehilytimoney.ie			Trial	Pit Lo	bg		TP/T	14
1	IMONEY	R2	1 XA66 Ireland				.				Sheet 1 o	f1
Project	Derrin	lough W	ind Farm		Projec	t No.	Coords (E,N):	6099	81.31	714057.28	Date	
					P1990)	Level:				13/08/20	19
Location	n:						Dimensions (m):				5cale	
Client:							Depth				Logged	1
						1	3.20				IH	
Water Strike	Depth	Type	Results/Sample F	Depth Ref ^(m)	Level (m)	Legend		Stratur	n Descri	ption		
				1.50			Grey silty S	SAND with occas	sional co	bbles and bould	ders	2
Remark	s: Modera	te water inf	low at 3.1m, samp	ple taken at 2.5m	l	1					Plant Used:	:

1		Fehily Tr Sir Bi R21	y Timoney & Co. he Grainstore Tel: +3 agletons Lane Emai agenalstown Co. Carlow www.fi 1 XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial P	it Log	TrialPit No TP/T15 Sheet 1 of 1
Project	Derrinlou		ind Farm		Proje	ct No.	Coords (E,N):	610807.05 714383.2	1 Date
Name:	Derninot	agii w			P199	D	Level:		13/08/2019
Location:							Dimensions		Scale
Client							Depth		Logged
						1	2.70		I IH
Water Strike	Depth	Type	Results/Sample Ref	. Depth (m)	Level (m)	Legend	0.0	Stratum Description	
						્યાદ સાહ સા ૬ સાંદ સાંદ સાંદ સાંદ સાં	Soft brown PEA	AI	
				0.20			Soft grey slight	ly sandy CLAY with occasional g	avel and
							cobbles littl at	In nore nequent bounders at 21	
									-
									-
									2 -
				2.70				End of Pit at 2.700m	
									3 -
									4 -
Remarks	: Moderate g	groundw	ater inflow at 2.6m sa	imple taken a	at 2m				Plant Used:

Т		Fehily Th Sir Ba (R21	y Timoney & Co. ne Grainstore Te ngletons Lane E agenalstown Co. Carlow w 1 XA66 Ireland	el: +353-59-9723800 Email: info@ftco.ie Web: ww.fehilytimoney.ie	Trial Pit			t Log		TrialPit No TP/T16 Sheet 1 of 1	5
Project	Derrinley		ind Form		Projec	ct No.	Coords (E,N):	610312.71	714731.09	Date	
Name:	Derninot	ign w	inu fann		P1990)	Level:			14/08/2019	
Location	:						Dimensions]	Scale	
							Depth			Logged	
							2.60			IH	
Water Strike	Sampl Depth	les & In S Type	Situ Testing Results/Sample F	Depth Ref ^(m)	Level (m)	Legend) /on coft dowl by	Stratum Descri	ption		
				0.50		site and a	Soft grey sandy subrounded cobt	SILT with frequent	subangular to s firm at 1.2m st	iff at 2m	
		2.6								1	2
Remarks	S: Moderate o	iroundw	ater inflow 2.4m.	2.60	: 2m. si	de collapsi	ng below 2m	End of Pit at 2.6	300m	3 Plant Lised:	
Remarks	: Moderate g	Iroundw	ater inflow 2.4m,	sample taken at	: 2m, si	de collapsi	ng below 2m			Plant Used:	

	FEHILY	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-59-9723 Email: info@ftco Web: www.fehilytimone				-	Trial F	Pit Log	g	TrialPit No TP/T17 Sheet 1 of 1
Project	Derrin				Project	No.	Coords (E,N):	610067	7.27 715971.53	Date
Name:	Derrir	liougn w	ind Farm		P1990		Level:			12/08/2019
Locatio	n:						Dimensions			Scale
							Depth			1:25
Client:							2.80			IH
Water Strike	S Depth	Type	Situ Testing Results/Sample I	Depth Ref ^(m)	Level (m)	Legend		Stratum D	Description	
				2.80			Firm grey slig cobbles 1.5m	EAT with frequen	t at 2.800m	bangular 1 -
Remark	s: Modera	ate groundw	vater inflow at 2.5	m, sample taken a	at 2m					Plant Used:

т	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland					·	Trial P	it Log		TrialPit No	8
Project	Derrinlo	ugh W	ind Farm		Projec	ct No.	Coords (E,N):	609950.32 7166	635.67	Date	
					P1990	0	Level:			13/08/2019 Scale	1
Location	:						(m):			1:25	
Client:							Depth 2.70			Logged IH	
Vater	Samp	les & In S	Situ Testing	Depth (m)	Level	Legend		Stratum Description			
	Depth	Type	Results/Sample Ref	2.70		silie silie silie silie silie silie silie silie silie silie silie silie silie silie silie silie silie silie	Soft brown and	dark brown PEAT y sandy CLAY (damp) grav subangular to subrounded End of Pit at 2.700m	relly below cobbles	v 1.5m,	
Remarks	Slow grour	ndwater	inflow at 2.6m, sampl	e taken at 2r	n	1				Plant Used:	

		Fehil Ti Si B R2	Tel: +353 Email: i \ www.feh	3-59-9723800 info@ftco.ie Web: nilytimoney.ie			Trial Pi	t Log		TrialPit f TP/T Sheet 1 c	No 19 of 1	
Project	Derri	nlough W	ind Farm			Project	No.	Coords (E,N):	609360.00	715723.00	Date	
Name:	Berri	nough H				P1990		Level:			08/10/20	019
Locatio	n:							Dimensions (m):			Scale 1:25	
Client:								Depth			Logge	d
ه ه	s	amples & In	Situ Testing		Denth			2.20			IH	
Wate Strik	Depth	Туре	Results/Samp	le Ref	(m)	Level (m)	Legend	Soft brown PEAT	Stratum Descr	iption		1
					2.20			Soft brown PEAI Firm to stiff grey s occasional subrou	End of Pit at 2.	200m	AY with	
Remark	s: Slow to	o moderate e taken at 1	groundwater in .2m	flow at 2	2.2m						Plant Used	1:

		Fehily Th Sir Ba (R21	y Timoney & Co. le Grainstore Tel: +3 gletons Lane Emai agenalstown Co. Carlow www.fr XA66 Ireland	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie	Trial Pit Log Project No. Coords (E,N): 609307.00 716406.				TrialPit N TP/T Sheet 1 o	lo 20 f 1
Project	Derrinlo	uah W	ind Farm		Projec	t No.	Coords (E,N): 609307.00	716406.00	Date	
Name:		- 3			P1990)	Level:		08/10/20	19
Locatio	n:						(m):		1:25	
Client:							Depth		Logged	
Water Strike	Samp Depth	oles & In S Type	Situ Testing Results/Sample Ref	Depth (m)	Level (m)	Legend	Stratum Descrip	tion		
				2.50		slife slife slife slife slife slife slife	Soft brown fibrous PEAT Firm grey sandy SILT with occasiona Occasional gravel and cobbles below End of Pit at 2.50	al subrounded h v 1.8m	boulders	1 - 2 - 3 -
Remar	ks: Groundwa	ter seep	age around boulders						Plant Used	:

	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-59-972: Email: info@ftcc Web: www.fehilytimone			53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial F	Pit Lo	bg		TrialPit TP/T Sheet 1	No 21 of 1		
Project							Projec	t No.	Coords (E,N):	6097	20.90	717111.31	Date	
Name:		errinlou	igh Wi	nd Farm			P1990)	Level:				13/08/20	019
Locatio	on:								Dimensions	ſ]	Scale)
									(m): Denth				1:25	d
Client:									2.80				IH	u
Water Strike	De	Sample epth	es & In S Type	itu Testing Results/Samp	le Ref	. Depth (m)	Level (m)	Legend		Stratum	ı Descrij	otion		
						2.80		alle alle alle a sile alle sile alle	Soft dark brow	dy CLAY, dam bulders at 2.3m	PEAT	ly below 1.5n	n	2
Remark	ks: M	loderate w	/ater infl	ow at 2.8m sa	ample ta	aken at 2m							Plant Used	d:
												L		
	FEHILY	Fehily Timoney & Co. The Grainstore Singletons Lane Bagenalstown Co. Carlow R21 XA66 Ireland Tel: +353-5 Email: infi Www.fehily						Trial P	it Lo	bg		TrialPit I TP/U Sheet 1 d	No JP of 1	
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Project	Derrinley		ind Form			Projec	t No.	Coords (E,N):	6078	86.69	714954.59	Date		
Name:	Derrinio	ugn w	ind Farm			P1990)	Level:				14/08/20	019	
Locatio	n:							Dimensions (m) [.]				Scale		
Client								Depth				Logge	d	
								3.50]	IH		
Water Strike	Depth	Type	Results/Samp	le Ref	Depth (m)	Level (m)	Legend	0.64	Stratun	n Descri	ption		1	
					1.10			Firm grey sandy	y slightly gra	avelly SI Pit at 3.5	LT 500m			
Remark	ks: Moderate (groundat	ter inflow at 3.3	Зm								Plant Used	- - - -	

		Fehily Th Sin Ba (R21	(Timoney & Co. e Grainstore gletons Lane agenalstown Co. Carlow XA66 Ireland	353-59-9723800 il: info@ftco.ie Web: fehilytimoney.ie		•	Trial P	it Log		TrialPit No TP05a Sheet 1 of 1	a
Project	Derrinlou	ugh Wi	ind Farm		Projec	ct No.	Coords (E,N):	605868.14	714315.48	Date	
indifie.		-			P1990)	Level:			15/08/2019)
Location	n:						(m):			1:25	
Client:							Depth			Logged	
	Samp	les & In S	Situ Testina				3.40			IH IH	
Wate Strike	Depth	Туре	Results/Sample Ref	Depth (m)	Level (m)	Legend		Stratum Des	cription		
				1.80		Alte shite s	Very soft dark b	rown and brown y sandy gravelly t ders End of Pit at	PEAT CLAY 2.4m - firm 3	3m -	2
Remark	s: Minor wate	r inflow	at 1.8m							Plant Used:	

	FEHILY	Fehily Th Sir Ba (R21	/ Timoney & Co. he Grainstore ngletons Lane agenalstown Co. Carlow I XA66 Ireland	Tel: +35 Email www.fe	53-59-9723800 : info@ftco.ie Web: ehilytimoney.ie		•	Trial P	it Lo	g		TrialPit I TPO Sheet 1 d	No 6a of 1
Project	Build					Projec	t No.	Coords (E,N):	60651	9.93	714632.34	Date	
Name:	Derrinio	ugn w	Ind Farm			P1990		Level:				15/08/20)19
Locatior	ו:							Dimensions	Г			Scale	
Client								Depth				Logge	d
			N.t					3.30	L			IH	
Water Strike	Depth	Туре	Results/Samp	ole Ref	Depth (m)	Level (m)	Legend	Very soft dark	Stratum brown and bro	Descri	otion AT		-
	Depth Type Results/Sample Ref (III)				1.70		Anne Anne Anne Anne Anne Anne Anne Anne	Soft grey sand cobbles	y gravelly CLA	AY with Pit at 3.3	occasional sub	prounded	2 -
													4
Remark	s: Moderate	 groundw	ater inflow at :	3.2m sa	mple taken a	at 2.2m						Plant Used	<u> </u> 1:

	FEHIL	1 .Y	Fehily Th Sin Ba (R21	Timoney & Co. e Grainstore gletons Lane igenalstown Co. Carlow XA66 Ireland	Tel: +38 Email www.fe	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial P	it Lo	bg		TrialPit I TP08 Sheet 1 d	No Ba of 1
Project		<u>E Y</u>					Projec	t No.	Coords (E,N):	6052	16.89	713911.43	Date	-
Name:	De	errinlou	igh Wi	nd Farm			P1990)	Level:				15/08/20)19
Locatio	on:								Dimensions	[Scale	
Client:									Depth				Logge	d
oliciti.		Sampl	00 8 10 9	itu Tostina					2.80	l			IH	
Water Strike	Dep	oth	Туре	Results/Samp	le Ref	Depth (m)	Level (m)	Legend	Vorus off shaded	Stratum	n Descrip	tion		
Remark	ks: Slo	ow to mo	derate g	roundwater in	flow at	1.10 2.80		sule sules s sules s	Soft to firm gre occasional sub	y slightly sar angular to su End of I	Pit at 2.80	elly CLAY with d cobbles and	Plant User	
Remark	ks: Slo	ow to mo	derate g	roundwater in	flow at	2.6m							Plant Usec	I:

		Fehil Ti Sii B	y Timoney & Co. he Grainstore ngletons Lane lagenalstown	Tel: +353-59-9723800 Email: info@ftco.ie Web:		•	Trial F	Pit Log	g	TrialPit No TP09	° a
1	FEHILY FIMONEY	R2	1 XA66 Ireland	www.ieniiyamoney.ie						Sheet 1 of	1
Project	Derri	1louah W	ind Farm		Projec	t No.	Coords (E,N):	607228.	.70 714024.93	B Date	
Name:		- J			P1990		Level:			16/08/201	19
Locatio	n:						Dimensions (m):			Scale	
Client							Depth			Logged	
			ou T 1				1.90			IH	
Water Strike	Depth	Type	Results/Sample	Ref (m)	Level (m)	Legend		Stratum D	escription		
				1.90			Soft grey sligh subangular to	ttly sandy gravel subrounded cob End of Pit	ly CLAY with occas bles and boulders at 1.900m	sional	2
Remark	s: Moder	ate groundw	vater inflow at 2.5	ōm sides unstable						Plant Used:	

	FEHILY	Y IY	Fehily Th Sin Ba (R21	r Timoney & Co. e Grainstore gletons Lane ggenalstown Co. Carlow XA66 Ireland	Tel: +3 Email www.fe	53-59-9723800 I: info@ftco.ie Web: ehilytimoney.ie			Trial F	Pit Lo	g		TrialP TP ² Sheet	iit No 10a 1 of 1
Project	Dei	rrinlou	iah Wi	nd Farm			Projec	t No.	Coords (E,N):	60683	0.81	713487.37	7 Da	te
Name:			.g				P1990		Level:				16/08	/2019
Locatio	n:								(m):	Γ			1:2	25
Client:									Depth				Log	ged H
Water Strike	Dept	Sample	es & In S Type	itu Testing Results/Samp	ble Ref	Depth (m)	Level (m)	Legend	0.00	Stratum	Descri	ption		
						1.20		sulte sulte	Very soft dark	brown and bro subrounded be End of P	Pelly CL oulders	AT AY with occa	isional	
Remark	ks: Moo	derate g	roundwa	ater inflow at 2	2.5m								Plant Us	sed:
												L		

		Fehily Tr Sir B R2 ²	y Timoney & Co. he Grainstore ngletons Lane agenalstown Co. Carlow 1 XA66 Ireland	Tel: +353-59-9723i Email: info@ftco. Web: www.fehilytimoney	300 ie v.ie		Trial I	Pit Log		TrialPit No TP13a Sheet 1 of 1
Project	Derrinlo	uah W	ind Farm		Pr	roject No.	Coords (E,N):	607039.52	712680.41	Date
Name:		agii II			P	1990	Level:			16/08/2019
Location	n:						Dimensions (m):			Scale 1:25
Client [.]							Depth			Logged
	Samr		Situ Tosting				2.60			IH
Water Strike	Depth	Туре	Results/Sample	e Ref	Lev (m	/el 1) Legend	Very soft bro	Stratum Desc	ription	
				1.75		alke alke a e alke alke a alke alke a e alke alke a alke alke al alke alke alke al alke alke alke al alke alke alke al alke alke alke alke alke al alke alke alke alke alke alke alke alke	Soft to firm of subangular	grey slightly sandy CL to subrounded cobbles	AY with occasion	al 2 -
				2.00				End of Pit at 2.	600m	
										3 -
										-
										4 -
Remark	s: Slow groui	ndwater	inflow at 2.4m,	Peat/Clay bou	ndary n	not horizontal				Plant Used:
									L	

т		Fehily Th Sir Ba (R21	/ Timoney & Co. le Grainstore Igletons Lane agenalstown Co. Carlow XA66 Ireland	Tel: +353 Email: ir V www.fehi	-59-9723800 nfo@ftco.ie Veb: ilytimoney.ie		•	Trial P	it Lo	g		Tria TP Shee	IPit No 1 61 et 1 of 1
Project	Derrinlou	uah W	ind Farm			Projec	ct No.	Coords (E,N):	609393	3.69	716168.52	2 [)ate
iname:		5				P1990)	Level:				12/0	8/2019
Location	:							(m):	Γ			1	:25
Client:								Depth 3 80				Lo	gged IH
Water Strike	Samp	les & In S Type	Situ Testing Results/Sample	e Ref	Depth (m)	Level (m)	Legend		Stratum	Descri	ption		
					1.10		Alle sille s	Soft dark and lig	y sandy SILT. cobbles and End of Pi	AT Damp rare bc	below 2m, al pulders	Iso gravelly	
Remarks	: Slow seepa	age at 2	8m, sample tak	ken at 2.	.5m							Plant I	Jsed:

		Fehily The Sing Bag C R21	Timoney & Co. e Grainstore Tel: +: letons Lane Ema genalstown o. Carlow www. XA66 Ireland	353-59-9723800 ill: info@ftco.ie Web: fehilytimoney.ie			Trial Pit	t Log		TrialPit N TP19 Sheet 1 o	lo)a of 1
Project	Derrinlo	uah Wii	nd Farm		Projec	t No.	Coords (E,N):	610115.43 7153	302.72	Date	
Name:	Derrino				P1990		Level:			14/08/20	19
Locatio	n:						Dimensions			Scale	
Client							Depth			Logged	1
							0.15			IH	
Water	Depth	Type	tu Testing Results/Sample Ref	- Depth (m) 0.15	Level (m)		Soft dark brown P Firm to stiff grey si subrounded cobbli boulders, hard dig	Stratum Description EAT ity SAND with occasion es and boulders below 1 End of Pit at 0.150m	al subang 1m - frequ	gular to Jent	
Remark	Pit dry, sa	mple take	n at 1m			<u> </u>				Plant Used:	:

	FEHILY TIMONEY	Fehily 1 The Sing Bag Co R21 2	Timoney & Co. Grainstore Tel: +3 letons Lane Ema jenalstown b. Carlow www.f KA66 Ireland	153-59-9723800 il: info@ftco.ie Web: fehilytimoney.ie			Trial Pit Log	TrialPit N TP20 Sheet 1 o	lo)a f 1
Project	Derrinlo	uah Wir	nd Farm		Projec	t No.	Coords (E,N): 609438.78 715001.22	Date	
Name:	Derrino				P1990)	Level:	13/08/20	19
Locatio	in:						Dimensions	Scale	
Client							Depth	Logged	
Client:				T T			2.60	ÎĤ	
Water Strike	Samp Depth	oles & In Sit	tu Testing Results/Sample Ref	Depth (m)	Level (m)	Legend	Stratum Description		
				0.20		ાં ગોદ ગોદ ગો ૬ ગોદ ગોદ ગોદ ગોદ ગો	Soft dark brown PEAT		-
						× × × × × × × × × × × × × × × × × × ×	Very soπ / grey SILI (MARL)		-
		0.50					Firm to stiff grey slightly sandy CLAY with occasional subangular to subrounded cobbles and rare boulders		2
									3
Remar	ks: Hard dig a	t 2.6m sar	mple taken at 2m	1		<u> </u>		Plant Used:	



Photo 1 Trial pit TP/T8



Photo 2 Trial pit TP/T10



Photo 3 Trial Pit TP/T13



Photo 4 Trial pit TP/T17



Photo 5 Excavated arisings from trial pit TP/T17



Photo 6 Trial pit SS/TP2



Photo 7 Trial pit TP/CSH2



Photo 8 Excavated arisings from trial pit TP/CSH2



Photo 9 Trial pit TP6a



Photo 10 Trial pit TP13a



Photo 11 Trial pit TP20a



Photo 12 Excavated arisings from trial pit BP1/TP1



Photo 13 Trial pit BP1/TP2



Photo 14 Trial pit BP3/TP2



Photo 15 Excavated arisings from trial pit BP3/TP2